AGENDA
CITY OF VALPARAISO & VALPARAISO CABLE AUTHORITY JOINT MEETING
465 Valparaiso Parkway
Valparaiso, Florida
850-729-5402
May 9, 2016
6:00 pm

Invocation (Comm. Wasdin)
Pledge of Allegiance (Mayor Arnold)

APPROVAL OF MINUTES
1. April 11, 2016
2. April 18, 2016

CITIZENS' CONCERNS (non-agenda items)
1. Resident
2. Non-resident

ACTION ITEMS/POTENTIAL ORDINANCES
1. Development Order Jasmine North Subdivision ----------Attach 1
2. City Audit
3. Etc.

OLD BUSINESS
1. Ordinance No. 671 Business Property Tax Exemption----------Attach 2
2. Dangerous Building Update
3. Etc.

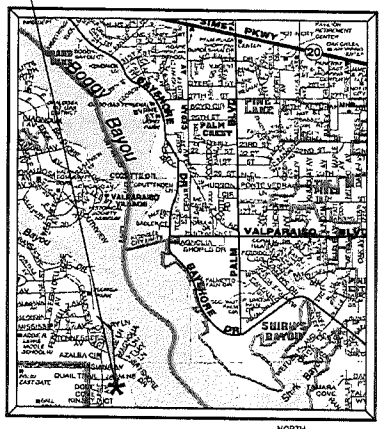
REPORTS / CORRESPONDENCE / ANNOUNCEMENTS
1. TPO/DOT
2. Stormwater
3. Legal Activities Update
4. Community Liaison Report
5. Construction Activity
6. Schedule Executive Session
7. Disbursements
8. Etc.

CONSTRUCTION PLANS FOR:

JASMINE NORTH SUBDIVISION

Niceville, Florida

PROJECT LOCATION



VICINITY MAP

NORTH

PREPARED FOR:

Valparaiso Realty Co.

P.O. Box 8
Valparaiso, Florida 32580
Phone: (850) 678-7812
Contact: Pat Byrne

DIRECTIONS TO LOCATE SITE:

LATITUDE = 30°29'15.93"N
LONGITUDE = 86°29'7.96"W
SECTION = 19
TOWNSHIP = 1 SOUTH
RANGE = 22 WEST
COUNTY = OKALOOSA
CITY = VALPARAISO

THE PROJECT SITE IS LOCATED IN CITY OF VALPARAISO, FLORIDA, APPROXIMATELY 0.19 MILES SOUTH OF THE INTERSECTION OF LOUISIANA AVENUE AND JASMINE AVENUE, ON EAST SIDE OF JASMINE AVENUE.

UTILITY COMPANIES:

COMPANY	PHONE NO.
GULF POWER	(850) 678-4115
VALPARAISO BROADBAND	(850) 729-5404
CENTURY LINK	(850) 664-3731
OKALOOSA GAS DISTRICT	(850) 729-4880
VALPARAISO FIRE DEPARTMENT	(850) 729-5410
VALPARAISO WATER/SEWER DEPARTMENT	(850) 729-5407
CITY OF VALPARAISO/PUBLIC WORKS	(850) 729-5402

DUTY TO INDEMNIFY

THE CONTRACTOR SHALL DEFEND, INDEMNIFY, KEEP AND SAVE HARMLESS THE OWNER AND ENGINEER AND THEIR RESPECTIVE MEMBERS, REPRESENTATIVES, AGENTS AND EMPLOYEES, IN BOTH INDIVIDUAL AND OFFICIAL CAPACITIES, AGAINST ALL SUITS, CLAIMS, DAMAGES, LOSSES AND EXPENSES, INCLUDING ATTORNEY'S FEES, CAUSED BY, GROWING OUT OF, OR INCIDENTAL TO THE PERFORMANCE OF THE WORK UNDER THE CONTRACT BY THE CONTRACTOR OR ITS SUBCONTRACTORS TO THE FULL EXTENT AS ALLOWED BY THE LAWS OF THE STATE OF FLORIDA AND NOT BEYOND ANY EXTENT WHICH WOULD RENDER THESE PROVISIONS VOID OR UNENFORCEABLE. IN THE EVENT OF ANY SUCH INJURY (INCLUDING DEATH) OR LOSS OR DAMAGE, OR CLAIMS THEREFORE, THE CONTRACTOR SHALL GIVE PROMPT NOTICE TO THE OWNER.

AUTHORITY AND RESPONSIBILITY

THE ENGINEER, AS REPRESENTATIVE OF THE OWNER, SHALL NOT GUARANTEE THE WORK OF ANY CONTRACTOR OR SUBCONTRACTOR. SHALL HAVE NO AUTHORITY TO STOP WORK, SHALL HAVE NO SUPERVISION OR CONTROL AS TO THE WORK OR PERSONS DOING THE WORK, SHALL NOT HAVE CHARGE OF THE WORK, SHALL NOT BE RESPONSIBLE FOR SAFETY IN, ON, OR ABOUT THE JOB SITE OR HAVE ANY CONTROL OF THE SAFETY OR ADEQUACY OF ANY EQUIPMENT, BUILDING COMPONENT, SCAFFOLDING, SUPPORTS, FORMS, OR OTHER WORK AIDS, AND SHALL HAVE NO DUTIES OR RESPONSIBILITIES IMPOSED BY THE STRUCTURAL WORK ACT.

INDEX OF SHEETS:

C00..... TITLE SHEET
C01..... EROSION CONTROL & DEMOLITION PLAN
C02..... SITE PLAN
C03..... GRADING & DRAINAGE PLAN
C04..... UTILITY PLAN
C05..... PLAN & PROFILE (ROAD "A & B")
C06..... PLAN & PROFILE (ROAD "B")
C07..... MISCELLANEOUS DETAILS
C08..... MISCELLANEOUS DETAILS
C09..... POTABLE WATER DETAILS
C10.... SANITARY SEWER DETAILS
C11.... SPECIFICATIONS
C12.... SPECIFICATIONS

NOTE:

USE LATEST STANDARD SPECIFICATIONS FOR DEVELOPERS - CITY OF VALPARAISO, FLORIDA) UNLESS OTHERWISE NOTED.

NOT RELEASED FOR CONSTRUCTION

JENKINS ENGINEERING, INC.
GCT ENGINEERING

JENKINS ENGINEERING, INC.
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MATTHEW N. JENKINS
CERTIFICATE No. 57642
STATE OF FLORIDA
PROFESSIONAL ENGINEER
MATTHEW N. JENKINS, P.E.
FL REGISTRATION NO. 57642

REV	DATE	DESCRIPTION	BY
1	04-21-16	REVISED PER CITY OF VALPARAISO COMMENTS DATED 04-21-16	
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VALPARAISO REALTY COMPANY

JASMINE NORTH SUBDIVISION
CITY OF VALPARAISO, FLORIDA

TITLE SHEET

ADVANCED ENGINEERING & ENGINEERING SPECIAL REQUIREMENTS

JOB: E15020
DATE: 04-06-2016
DESIGNED: MS/MZ
DRAWN: MS

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0 1"
IF NOT ONE INCH ON THIS SHEET ADJUST SCALES ACCORDINGLY

DRAWING NUMBER
1 OF 13

SHEET NUMBER
C00

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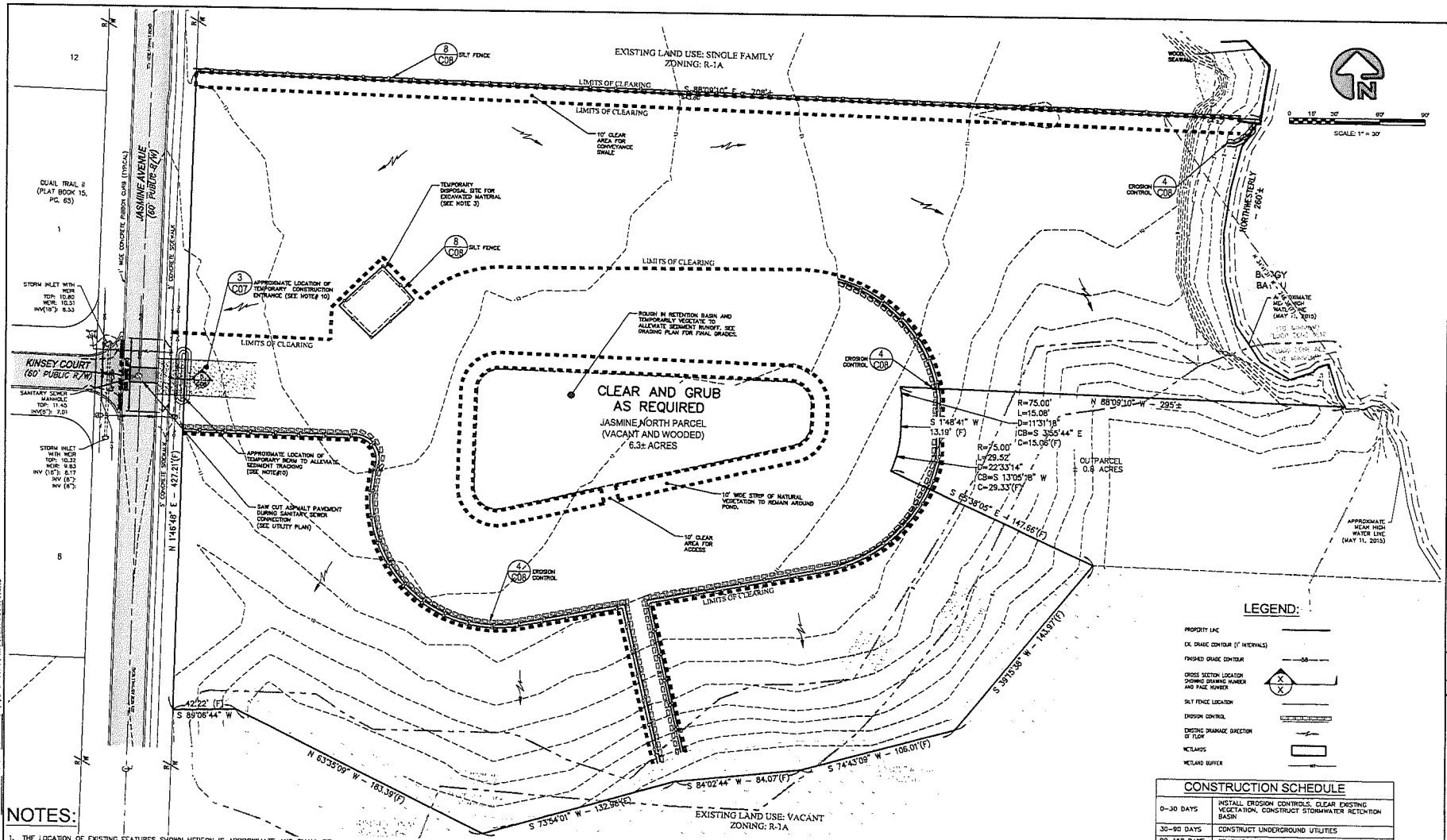
Last plotted by: Marek Scotka Plot Style: GCT2011.ctb Plot Scale: 1:2.58 Plot Date: 5/4/2016 9:32 AM Plotter used: None



LEGEND:

- PROPERTY LINE
- EX. GRADE CONTOUR (1' INTERVALS)
- FINISHED GRADE CONTOUR — 58 —
- CROSS SECTION LOCATION SHOWING DRAWING NUMBER AND PAGE NUMBER
- SILT FENCE LOCATION
- EROSION CONTROL
- EXISTING DRAINAGE DIRECTION OF FLOW
- WETLANDS
- WETLAND BUFFER

CONSTRUCTION SCHEDULE

0–30 DAYS	INSTALL EROSION CONTROLS, CLEAR EXISTING VEGETATION, CONSTRUCT STORMWATER RETENTION BASIN
30–90 DAYS	CONSTRUCT UNDERGROUND UTILITIES
90–150 DAYS	GRADING AND CONSTRUCTION OF DRIVE
150–180 DAYS	COMPLETION – STABILIZE SITE, TESTING, FINAL CLEANUP, REMOVAL OF TEMPORARY SILT FENCES

– PRIOR TO CLEARING, SILT FENCING AND EROSION CONTROL SHALL BE INSTALLED ACCORDING TO DETAILS 8/C08 AND 4/C08 OF THE CONSTRUCTION PLANS. THE LOCATIONS ARE DEPICTED ON DWG NO. 1 AND 3 OF THE CONSTRUCTION PLANS.

– AFTER THE CLEARING AND GRUBBING OF THE SITE, CONSTRUCTION OF THE RETENTION BASIN, AND A ROUGH FINISHED GRADE IS ESTABLISHED, ALL AREAS THAT HAVE BEEN DISTURBED SHALL BE STABILIZED WITH GRASS APPLIED AT MANUFACTURER'S RECOMMENDATIONS. ALL SLOPES 4:1 OR GREATER SHALL BE STABILIZED WITH SOD AS INDICATED IN THE CONSTRUCTION PLANS.

– AS INLETS OR MANHOLES ARE INSTALLED ON SITE, THEY ARE TO BE PROTECTED FROM EROSION AND SEDIMENT RUNOFF BY THE USE OF FILTER FABRIC AND PROPERLY INSTALLED SILT FENCING AS PER DETAIL 7/C08.

– ALL INSTALLATION OF INLETS, PAVING, ROADWAYS, AND EROSION CONTROL DEVICES SHALL CONFORM TO THE APPROVED CIVIL ENGINEERING CONSTRUCTION PLANS.

NOTES:

1. THE LOCATION OF EXISTING FEATURES SHOWN HEREON IS APPROXIMATE AND SHALL BE VERIFIED IN THE FIELD PRIOR TO BEGINNING CONSTRUCTION.
2. THE CONTRACTOR SHALL COORDINATE THE RELOCATION OF ALL UTILITIES WITH EACH UTILITY COMPANY.
3. ALL EXCESS SOIL MATERIAL, IF ANY, SHALL BE STORED ON-SITE IN THE LOCATION SHOWN FOR LATER USE OR REMOVED FROM THE SITE AND DISPOSED OF PROPERLY IN A MANNER APPROVED BY THE CITY OF VALPARAISO. LOCATION OF TEMPORARY DISPOSAL SITE TO BE FIELD DETERMINED BY CONTRACTOR AND CAN SHIFT TO ACCOMMODATE CONSTRUCTION SEQUENCING AS LONG AS PROPER EROSION CONTROLS ARE MAINTAINED.
4. AN FDEP NPDES (NATIONAL POLLUTION DISCHARGE ELIMINATION SYSTEM) PERMIT IS REQUIRED FOR THIS PROJECT AREA AND IS THE RESPONSIBILITY OF THE CONTRACTOR. VERIFY WITH THE CITY OF VALPARAISO THAT THE PERMIT HAS BEEN OBTAINED PRIOR TO COMMENCEMENT OF SITE WORK.
5. CONTRACTOR TO ERECT SILT/SEDIMENTATION CONTROLS, AS DEPICTED HEREIN, PRIOR TO COMMENCEMENT OF CONSTRUCTION. ALL EROSION CONTROL MEASURES, INCLUDING PERIMETER CONTROLS, SITE ENTRANCE, INLET PROTECTION, AND SEDIMENT BASINS ARE TO BE REGULARLY INSPECTED AND MAINTAINED THROUGHOUT THE COURSE OF CONSTRUCTION. IN ADDITION TO THE NPDES PERMITTEE'S INSPECTIONS, THE CITY OF VALPARAISO WILL ALSO INSPECT AND MONITOR THE EROSION CONTROL MEASURES AND PROVIDE RESULTS TO THE NPDES PERMITTEE.
6. THE CONTRACTOR SHALL VERIFY THAT ALL APPLICABLE PERMITS HAVE BEEN OBTAINED PRIOR TO COMMENCEMENT OF CONSTRUCTION.
7. TOPOGRAPHIC INFORMATION BY GUSTIN, COTHERN, & TUCKER, INC., BOUNDARY AND TOPOGRAPHIC SURVEY, DATED 09/01/15. ELEVATIONS SHOWN HEREON ARE REFERENCED TO THE NATIONAL GEODETIC VERTICAL DATUM OF 1988.
8. IF THE SITE CONTRACTOR IS THE SOLE NPDES PERMITTEE AND THE PERMIT IS CLOSED UPON COMPLETION OF THE SITE WORK, IT WILL BE THE RESPONSIBILITY OF THE DEVELOPER TO INSURE THAT EACH INDIVIDUAL BUILDER PROVIDES ADEQUATE EROSION CONTROL DURING LOT CLEARING AND CONSTRUCTION ACTIVITIES.
9. THE CITY WILL VERIFY THAT NPDES WEEKLY INSPECTIONS ARE CONDUCTED. THE CONTRATOR/DEVELOPER WILL PROVIDE UPON REQUEST BY THE CITY COPIES OF THE NPDES INSPECTION SHEETS OR MAKE THEM ACCESSIBLE ON SITE.
10. LOCATION OF CONSTRUCTION ENTRANCE AND BERM TO BE FIELD DETERMINED BY CONTRACTOR AND CAN SHIFT ALONG PROPERTY LINE TO ACCOMMODATE CONSTRUCTION SEQUENCING.
11. ANY SILT FENCE SHOWN OUTSIDE OF PROPERTY LINE IS FOR ILLUSTRATIVE PURPOSES ONLY. SILT FENCE TO BE INSTALLED INSIDE OR ALONG PROPERTY BOUNDARY.
12. REMOVAL OF EXISTING CURB AND CUTTER AND SIDEWALK SHALL BE TO THE NEAREST CONSTRUCTION JOINT.

NOT RELEASED FOR CONSTRUCTION

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MATTHEW H. ZIMMER CERTIFICATE No. 57642 STATE OF FLORIDA PROFESSIONAL ENGINEER

MATTHEW H. ZIMMER, P.E.
FL REGISTRATION NO. 57642

REV	DATE	DESCRIPTION	BY
1	05-05-16	REVISED PER CITY OF VALPARAISO COMMENTS (04-27-16)	
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VALPARAISO REALTY COMPANY

JASMINE NORTH SUBDIVISION
CITY OF VALPARAISO, FLORIDA

EROSION CONTROL & DEMOLITION PLAN

J:\E15020\E15020 JASMINE NORTH SUBDIVISION\DWG\E15020 JASMINE NORTH SUBDIVISION.DWG

JOB: E15020
DATE: 04-06-2016
DESIGNED: MS/MZ
DRAWN: MS

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IF NOT ONE INCH ON THIS SHEET, ADJUST SCALES ACCORDINGLY

DRAWING NUMBER
2 OF 13

SHEET NUMBER
C01

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Last plotted by: Mark Scotka Plot Style: GCT2011.ctb Plot Scale: 1:2.58 Plot Date: 5/4/2016 9:33 AM Plotter used: None

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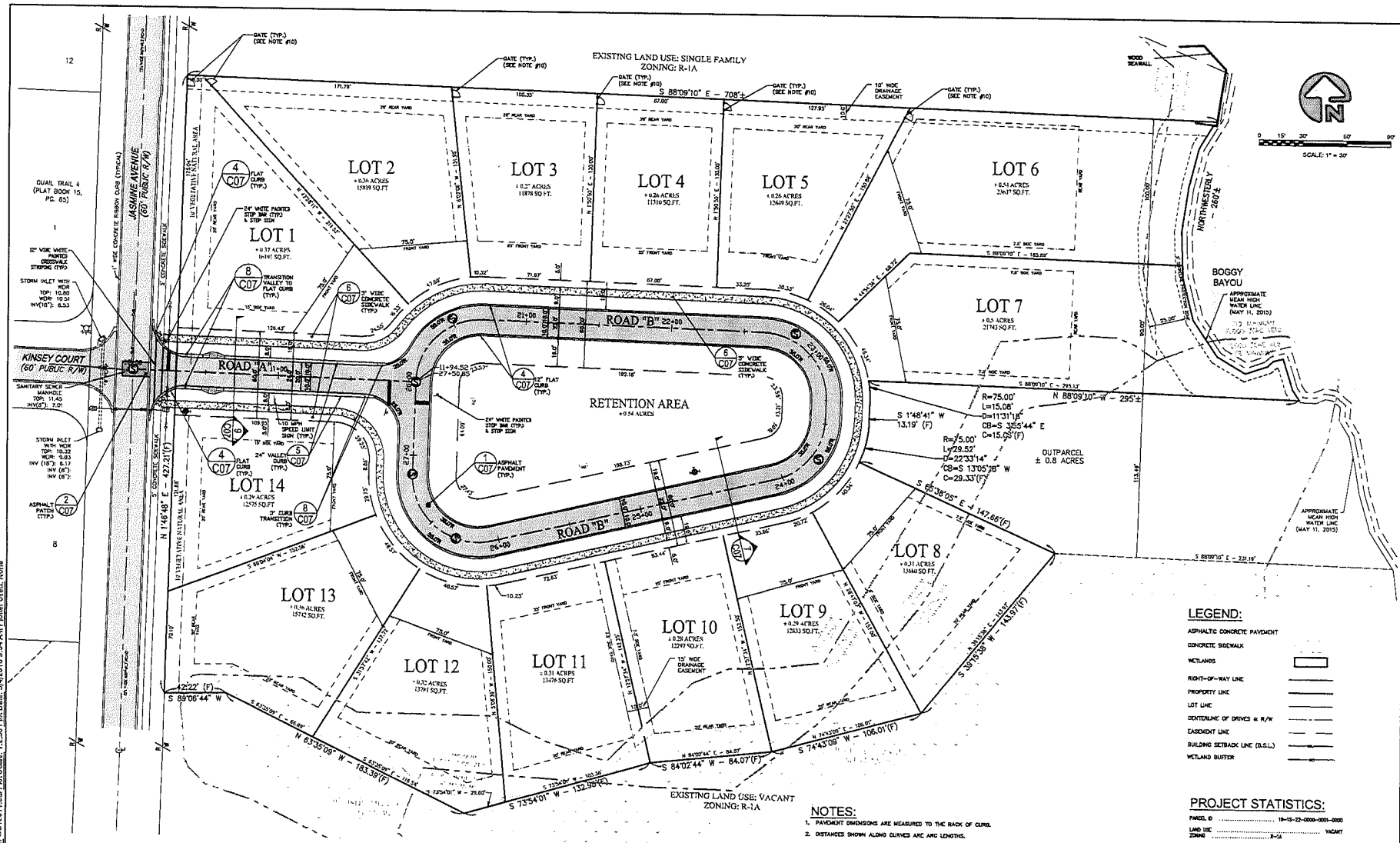

LEGEND:

- ASPHALTIC CONCRETE PAVEMENT
- CONCRETE SIDEWALK
- WETLANDS
- RIGHT-OF-WAY LINE
- PROPERTY LINE
- LOT LINE
- CENTERLINE OF DRIVES & R/W
- EASEMENT LINE
- BUILDING SETBACK LINE (B.S.L.)
- WETLAND BUFFER

PROJECT STATISTICS:

PARCEL ID	19-15-22-0000-0001-0000
LAND USE	VACANT
ZONING	R-1A
PROJECT SITE AREA	8.80± AC
DESCRIPTION	14 SINGLE FAMILY LOTS
PROVIDED DENSITY	2.08 UNITS/ACRE
PROVIDED BUILDING HEIGHT	35'
REQUIRED SETBACKS:	
FRONTAGE YARD	25'
SIDE YARD	7.5'
REAR YARD	20'
REQUIRED MINIMUM LOT SIZE	11,250 S.F.
REQUIRED MINIMUM LOT WIDTH	75'
MAXIMUM LOT COVERAGE	40%
MAXIMUM BUILDING HEIGHT	35'

NOTES:

1. PAVEMENT DIMENSIONS ARE MEASURED TO THE BACK OF CURB.
2. DISTANCES SHOWN ALONG CURVES ARE ARC LENGTHS.
3. RETURN RADIUS IS 25' UNLESS OTHERWISE DIMENSIONED.
4. THIS PARCEL LIES IN FLOOD ZONES X (NO MINIMUM) AND AE7&8 (7' AND 8' MINIMUM FLOOR ELEVATIONS) AND IN WL 10 (10' MINIMUM FLOOR ELEVATION) AS DETERMINED BY SCALE FROM FEMA FLOOD PANEL 12091C0476H, DATED 12/8/2002.
5. CONCRETE SIDEWALKS WILL BE CONSTRUCTED AT TIME OF HOME CONSTRUCTION BY THE BUILDING CONTRACTOR. SIDEWALK WILL BE INSTALLED ON ENTIRE ROAD FRONTAGE TO LOT LIMITS.
6. THE DEVELOPER WILL BE RESPONSIBLE FOR INSTALLATION OF ALL SIDEWALK SEGMENTS OUTSIDE OF PROPERY LIMITS, AS SHOWN HEREON, INCLUDING CONNECTION TO EXISTING CITY SIDEWALKS.
7. ALL STREET STRIPING WILL BE THE RESPONSIBILITY OF THE DEVELOPER, INCLUDING THE CROSS WALKS AND STOP BARS AT ENTRANCE.
8. THE DEVELOPER SHALL BE RESPONSIBLE FOR PROVIDING AND INSTALLING ALL SIGNAGE, INCLUDING STREET, STOP, AND SPEED LIMIT SIGNAGE. THE CITY WILL NOT BE RESPONSIBLE FOR CUSTOM SIGNAGE. IF FUTURE REPLACEMENT IS REQUIRED, THE CITY WILL ONLY REPLACE SIGNAGE PER MUTCD STANDARDS.
9. THE VEGETATIVE PRESERVATION AREA SHOWN IS INTENDED TO PROVIDE A NATURAL SCREENING BETWEEN THE RESIDENTS AND ADJACENT ROAD. IF THE VEGETATIVE BUFFER WILL BE DEEMED TO THE CITY, ALL DEAD AND DAMAGED TREES MUST BE REMOVED PRIOR TO CITY ACCEPTANCE.
10. DURING INSTALLATION OF FENCE GATE MUST BE INSTALLED WITHIN DRAINAGE EASEMENT TO ALLOW ACCESS BY CITY OF VALPARAISO TO CONVEYANCE SWALE.

NOT RELEASED FOR CONSTRUCTION

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MATTHEW H. JENKINS, P.E.
FL REGISTRATION NO. 57642

REV	DATE	DESCRIPTION	BY
1	06-03-16	KEY SUBMITTED TO VALPARAISO CONTACTS DATED 5-31-16	
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VALPARAISO REALTY COMPANY

JASMINE NORTH SUBDIVISION
CITY OF VALPARAISO, FLORIDA

SITE PLAN

JOB: E15020
DATE: 04-06-2016
DESIGNED: MS/MZ
DRAWN: MS

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3 OF 13

SHEET NUMBER
C02

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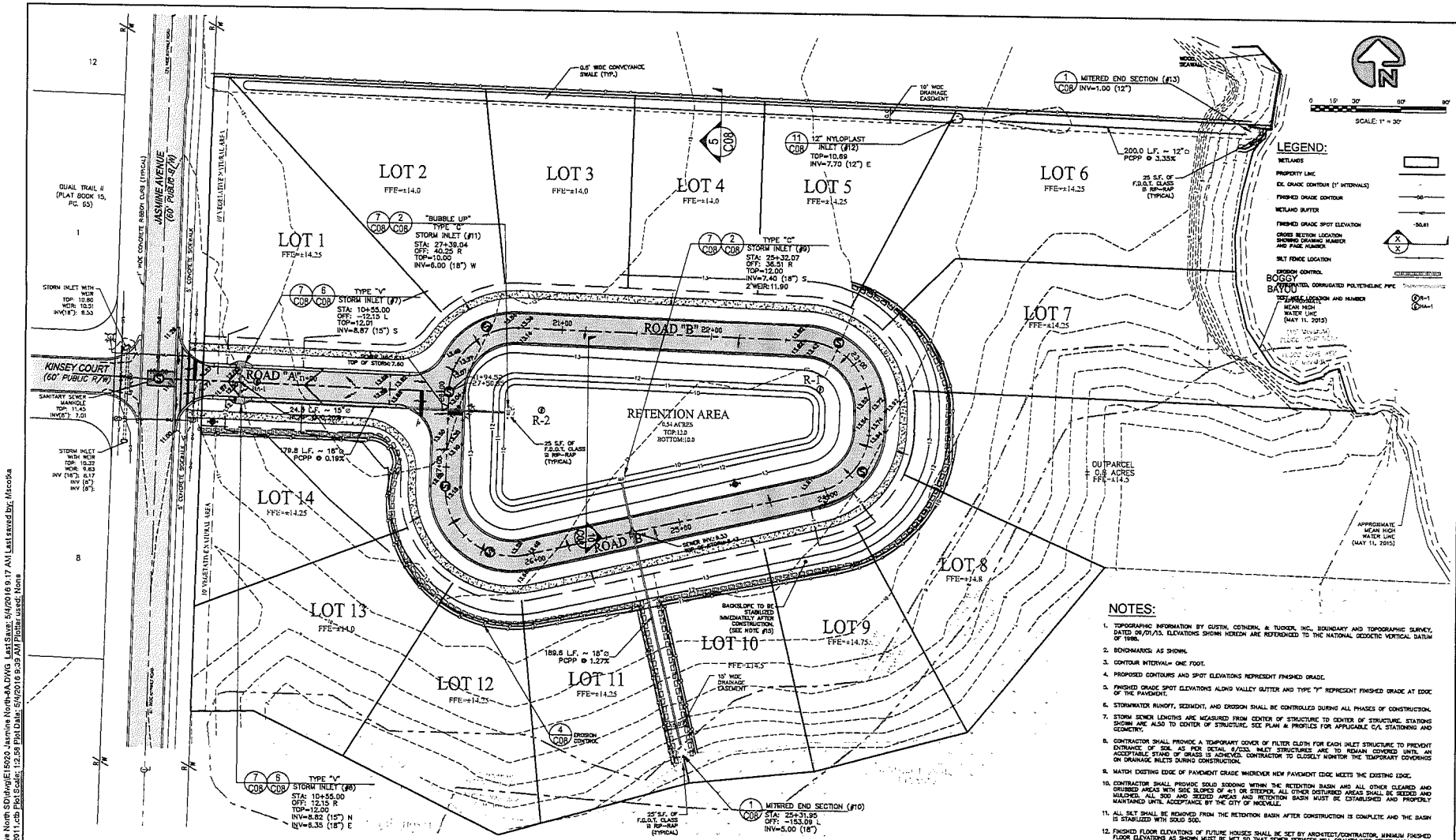


LEGEND:

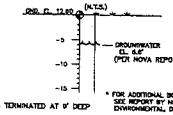
- WETLANDS
- PROPERTY LINE
- EX. GRADE CONTOUR (1' INTERVALS)
- FINISHED GRADE CONTOUR
- WETLAND BUFFER
- FINISHED GRADE SPOT ELEVATION
- CROSS SECTION LOCATION SHOWING DRAWING NUMBER AND PAGE NUMBER
- SILT FENCE LOCATION
- EROSION CONTROL
- PERFORATED, CORRUGATED POLYETHELENE PIPE
- TOP/INVERT LOCATION AND NUMBER
- APPROXIMATE MEAN HIGH WATER LINE (MAY 11, 2013)

NOTES:

1. TOPOGRAPHIC INFORMATION BY GUSTIN, COTHERN, & TUCKER, INC., BOUNDARY AND TOPOGRAPHIC SURVEY, DATED 06/01/13. ELEVATIONS SHOWN HEREON ARE REFERENCED TO THE NATIONAL GEODETIC VERTICAL DATUM OF 1988.
2. BENCHMARKS: AS SHOWN.
3. CONTOUR INTERVAL= ONE FOOT.
4. PROPOSED CONTOURS AND SPOT ELEVATIONS REPRESENT FINISHED GRADE.
5. FINISHED GRADE SPOT ELEVATIONS ALONG VALLEY GUTTER AND TYPE "F" REPRESENT FINISHED GRADE AT EDGE OF THE PAVEMENT.
6. STORMWATER RUNOFF, SEDIMENT, AND EROSION SHALL BE CONTROLLED DURING ALL PHASES OF CONSTRUCTION.
7. STORM SEWER LENGTHS ARE MEASURED FROM CENTER OF STRUCTURE TO CENTER OF STRUCTURE. STATIONS SHOWN ARE ALSO TO CENTER OF STRUCTURE. SEE PLAN & PROFILES FOR APPLICABLE C/L STATIONING AND GEOMETRY.
8. CONTRACTOR SHALL PROVIDE A TEMPORARY COVER OF FILTER CLOTH FOR EACH INLET STRUCTURE TO PREVENT ENTRANCE OF SOIL AS PER DETAIL 6/C03. INLET STRUCTURES ARE TO REMAIN COVERED UNTIL AN ACCEPTABLE STAND OF GRASS IS ACHIEVED. CONTRACTOR TO CLOSELY MONITOR THE TEMPORARY COVERINGS ON DRAINAGE INLETS DURING CONSTRUCTION.
9. MATCH EXISTING EDGE OF PAVEMENT GRADE WHEREVER NEW PAVEMENT EDGE MEETS THE EXISTING EDGE.
10. CONTRACTOR SHALL PROVIDE SOLID SODDING WITHIN THE RETENTION BASIN AND ALL OTHER CLEARED AND GRUBBED AREAS WITH SIDE SLOPES OF 4:1 OR STEEPER. ALL OTHER DISTURBED AREAS SHALL BE SEEDED AND MULCHED. ALL SOD AND SEEDED AREAS AND RETENTION BASIN MUST BE ESTABLISHED AND PROPERLY MAINTAINED UNTIL ACCEPTANCE BY THE CITY OF NICEVILLE.
11. ALL SILT SHALL BE REMOVED FROM THE RETENTION BASIN AFTER CONSTRUCTION IS COMPLETE AND THE BASIN IS STABILIZED WITH SOLID SOD.
12. FINISHED FLOOR ELEVATIONS OF FUTURE HOUSES SHALL BE SET BY ARCHITECT/CONTRACTOR. MINIMUM FINISHED FLOOR ELEVATIONS AS SHOWN MUST BE MET SO THAT SEWER SERVICES WILL GRAVITY INTO SEWER MAIN.
13. IF SOIL CONDITIONS ARE NOT CONSISTENT WITH SOIL BORINGS, OR IF MUCK IS ENCOUNTERED, THE CONTRACTOR SHALL NOTIFY THE ENGINEER.
14. CONTRACTOR TO BE ADVISED THAT GROUNDWATER COULD BE ENCOUNTERED DURING CONSTRUCTION OF THE PROJECT. IF DEWATERING WERE TO BE REQUIRED, COMPLETE DETAILS OF TEMPORARY DIKES, DISCHARGE LOCATIONS, TIME PERIODS, AND METHODS FOR PREVENTING TURBID DISCHARGES WILL BE PROVIDED TO THE REQUIRED AGENCIES. SEE REPORT BY NOVA ENGINEERING ENVIRONMENTAL, DATED JULY 07, 2015.
15. ALL PROPOSED BERMS OR BASIN BACKSLOPES SHALL BE STABILIZED WITH SOLID SOD IMMEDIATELY UPON COMPLETION OF GRADING.
16. THE CONTRACTOR IS REQUIRED TO REVIEW THE COMPLETE NWFWMD PERMIT PRIOR TO CONSTRUCTION COMMENCEMENT.
17. AN 8 1/2 BY 11 WEATHER RESISTANT SIGN, INCLUDING THE NWFWMD PERMIT NUMBER SHALL BE PLACED ON THE PROPERTY FACING THE ROAD.
18. ALL OF THE DRAINAGE PIPE WILL BE CORRUGATED PERFORATED POLYETHYLENE PIPE THROUGH OUT THE PROJECT, UNLESS SPECIFIED OTHERWISE.
19. RESIDENTIAL DWELLING SHALL BE CONSTRUCTED WITH THE LOWEST FLOOR ELEVATION (EXCLUDING BASEMENTS AND NON-HABITABLE GARAGES) NOT LESS THAN NINE INCHES (9") ABOVE THE AVERAGE ACTUAL CROWN OR CENTER LINE OF THE LOWEST ELEVATION OF THE ROAD ABUTTING THE SUBJECT PROPERTY. EXEMPTIONS TO THIS REQUIREMENT MAY OCCUR ONLY WHERE AN UNDUE HARDSHIP DUE TO THE TERRAIN OF THE LAND EXISTS AND IMPROVEMENTS ARE APPROVED BY THE BUILDING OFFICIAL AND/OR DIRECTOR OF PUBLIC WORKS.
20. CONTRACTOR TO NOTE THAT TYPE "J" BOTTOM WILL BE REQUIRED FOR MAJORITY OF THE INLET STRUCTURES, BUT A STANDARD BOTTOM MATRIX SUBSTITUTED WHERE PIPE DIAMETERS AND ANGLES ALLOW. CONTRACTOR TO PROVIDE SHOP DRAWINGS OF STRUCTURES FOR APPROVAL PRIOR TO ORDERING.

FOR CONSTRUCTION

TEST HOLE DATA #R-1:

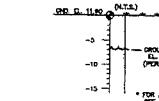


BORING TERMINATED AT 9' DEEP

* FOR ADDITIONAL BORING INFORMATION, SEE REPORT BY NOVA ENGINEERING AND ENVIRONMENTAL, DATED JULY 14, 2015

SAT. VERT. INFILTRATION RATE: (SEE REPORT)
GROUNDWATER: 6.6' (69 INCH) (SEE REPORT)
TEST DATE: 07/07/2015

TEST HOLE DATA #R-2:



BORING TERMINATED AT 9' DEEP

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SAT. VERT. INFILTRATION RATE: (SEE REPORT)
GROUNDWATER: 6.6' (69 INCH) (SEE REPORT)
TEST DATE: 07/07/2015

TYPICAL LOT DRAINAGE DETAIL

LOT DRAINAGE NOTES:
1. OWNER SHALL INFORM EACH LOT OWNER OF THEIR RESPONSIBILITY TO PROVIDE EROSION CONTROL DURING DEVELOPMENT OF EACH LOT AS SHOWN.
2. SILT FENCING SHOWN WITHIN LOTS SHALL BE PROVIDED DURING INDIVIDUAL HOME CONSTRUCTION.
3. FINISHED FLOOR ELEVATIONS OF FUTURE HOUSES SHALL BE SET BY CONTRACTOR SO THAT SEWER SERVICES WILL GRAVITY INTO SEWER MAIN, UNLESS IF ARRIVE UNDER OF ROAD.
4. ALL ROOF DRAINAGE AND ROOF RUNOFF SHALL BE DIRECTED TO STORMWATER COLLECTION SYSTEM ALONG THE ROADWAY.

JENKINS ENGINEERING, INC.
GCT ENGINEERING

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MATTHEW H. WINKLE CERTIFICATE No. 57642 STATE OF FLORIDA PROFESSIONAL ENGINEER

MATTHEW H. WINKLE, P.E.
FL REGISTRATION NO. 57642

REV	DATE	DESCRIPTION	BY
1	08-05-15	REVISED TO CITY OF VALPARAISO DEV. SUITE (PLOTTED 8-5-15)	
2			
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VALPARAISO REALTY COMPANY

JASMINE NORTH SUBDIVISION
CITY OF VALPARAISO, FLORIDA

GRADING & DRAINAGE PLAN
NOT VALID UNLESS SEALED & SIGNED WITH ORIGINAL SEAL & SIGNATURE

JOB: E15020
DATE: 04-06-2015
DESIGNED: MS/MZ
DRAWN: MS

BAR IS ONE INCH ON ORIGINAL
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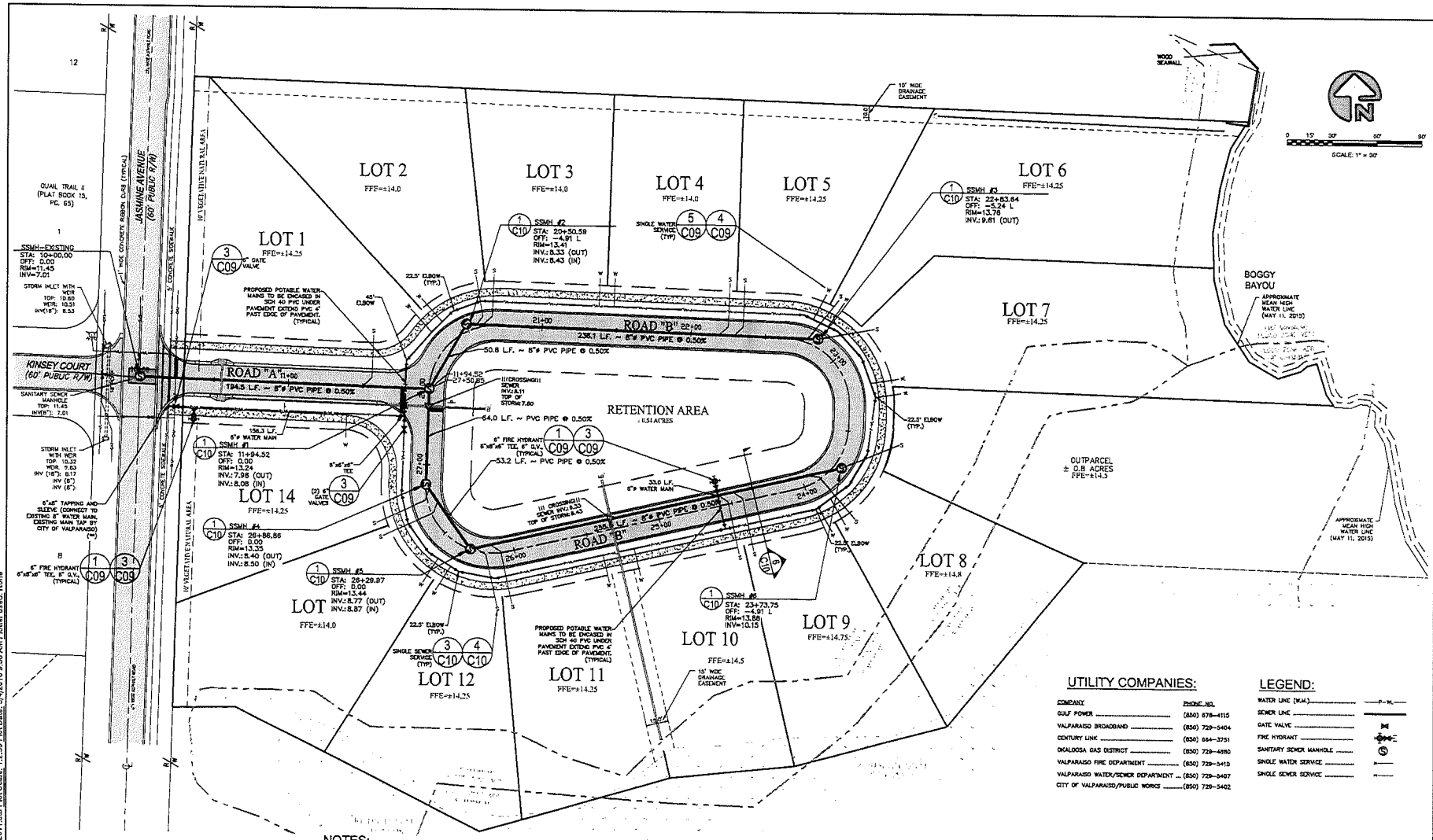
4 OF 13

SHEET NUMBER
C03



UTILITY COMPANIES:

COMPANY	PHONE NO.
GULF POWER	(850) 678-4115
VALPARAISO BROADBAND	(850) 729-5404
CENTURY LINK	(850) 654-3751
OKALOOSA GAS DISTRICT	(850) 729-4880
VALPARAISO FIRE DEPARTMENT	(850) 729-5410
VALPARAISO WATER/SEWER DEPARTMENT	(850) 729-5407
CITY OF VALPARAISO/PUBLIC WORKS	(850) 729-5402

LEGEND:

- WATER LINE (W.M.) — P-W-
- SEWER LINE
- GATE VALVE
- FIRE HYDRANT
- SANITARY SEWER MANHOLE
- SINGLE WATER SERVICE
- SINGLE SEWER SERVICE

NOTES:

1. THE LOCATION AND ELEVATION OF UTILITIES IS APPROXIMATE AND SHALL BE VERIFIED IN THE FIELD PRIOR TO BEGINNING CONSTRUCTION. IF THE LOCATION OR ELEVATION IS SUBSTANTIALLY DIFFERENT FROM THAT SHOWN ON THE PLANS OR IF CONFLICTS EXIST, THE ENGINEER SHALL BE NOTIFIED.
2. THE CONTRACTOR SHALL HAVE ALL APPLICABLE FLORIDA DEPARTMENT OF ENVIRONMENTAL PROTECTION AND LOCAL PERMITS PRIOR TO BEGINNING CONSTRUCTION.
3. TAPS TO EXISTING UTILITIES SHALL BE COORDINATED WITH THE LOCAL UTILITY CO. & THE CITY OF VALPARAISO. ALL TAPS SHALL BE MADE IN AN EXPEDITIOUS MANNER TO MINIMIZE INTERRUPTION OF SERVICE.
4. ALL WATER SERVICE PIPING SHALL BE PVC UNLESS OTHERWISE NOTED OR REQUIRED. A MINIMUM COVER OF 36" SHALL BE PROVIDED OVER THE WATER SERVICE. INSTALL TRACER WIRE ABOVE WATER SERVICE AND VALVES.
5. SEWER MAIN LENGTHS ARE MEASURED FROM CENTER OF MANHOLE TO CENTER OF MANHOLE.
6. SEWER MANHOLE STATIONS ARE TO CENTER OF MANHOLE.
7. WATER MAIN LENGTHS ARE MEASURED FROM FITTING TO FITTING.
8. ALL POTABLE WATER STRUCTURES AND SERVICES DEPICTED IN THE UTILITY LEGEND ALONG WITH THEIR DETAILS ARE LOCATED ON THE UTILITY DETAILS SHEET.
9. ALL SANITARY SEWER STRUCTURES AND SERVICES DEPICTED IN THE UTILITY LEGEND ALONG WITH THEIR DETAILS ARE LOCATED ON THE UTILITY DETAILS SHEET.
10. WATER MAINS CROSSING OVER SANITARY SEWER MAINS SHALL BE LAID TO PROVIDE A MINIMUM VERTICAL DISTANCE OF 18 INCHES BETWEEN THE INVERT OF THE UPPER PIPE AND THE CROWN OF THE LOWER PIPE.
11. WATER METER SYMBOLS SHOWN ON PLANS ARE NOT TO SCALE. EXACT LOCATION MAY BE ADJUSTED TO AVOID POTENTIAL CONFLICTS.
12. EXCAVATION AND BACKFILLING OF WATER MAIN TRENCHES SHALL CONFORM TO SPECIFICATIONS. PROVIDE A MINIMUM OF 36 INCHES OF COVER OVER WATER MAINS AND A MAXIMUM DEPTH OF 48 INCHES. DEPTHS BEYOND 48 INCHES MUST BE PRE APPROVED BY THE CITY OF VALPARAISO.
13. TESTING OF THE SANITARY SEWER SYSTEM IS REQUIRED PRIOR TO PAVING STREETS.
14. COORDINATE WITH THE CITY OF VALPARAISO FOR ALL INSPECTIONS. A FORTY EIGHT (48) HOUR NOTICE IS REQUIRED.
15. WHERE CONFLICTS OCCUR, A MINIMUM VERTICAL DISTANCE OF 18 INCHES BETWEEN THE INVERT OF THE UPPER PIPE AND THE CROWN OF THE LOWER PIPE SHALL BE MAINTAINED.
16. EXTEND WATER & SEWER TO BUILDING AS INDICATED. SITE CONTRACTOR TO COORDINATE CONTINUATION OF SERVICES WITH PLUMBING CONTRACTOR. OWNER/BUILDING CONTRACTOR TO COORDINATE THE NUMBER OF METERS FOR EACH BUILDING AND THEIR LOCATION WITH THE CITY OF VALPARAISO WATER AND SEWER DEPARTMENT.
17. OTHER UTILITIES SUCH AS CABLE, ELECTRIC, GAS, & TELEPHONE SHALL BE ENCASED IN PVC SLEEVES UNDER PAVEMENT. THESE UTILITIES SHALL NOT BE IN THE SAME DITCH AS WATER & SEWER LINES. NOR INTERFERE WITH THEIR DESIGNS FOR THESE SERVICES TO BE PROVIDED BY EACH INDIVIDUAL UTILITY AND REQUESTED/COORDINATED BY THE OWNER.
18. ALL FORCE MAINS, WATER MAINS, AND SERVICES SHALL BE ENCASED IN PVC SLEEVE WHEN CROSSING UNDER PAVEMENT, UNLESS OTHERWISE NOTED.
19. THE CONTRACTOR TO MAINTAIN ADEQUATE COVER OVER ANY EXISTING UTILITIES THAT WILL NOT BE RELOCATED.
20. DEVELOPER WILL PROVIDE A LIGHTING PLAN WHEN IT BECOMES AVAILABLE. ALL STREET LIGHTING FIXTURES WILL BE CUT OR SHIELDED TO PREVENT UPWARD GLARE.
21. THE CITY OF VALPARAISO WATER DEPARTMENT IS THE SOLE AGENCY ALLOWED TO MAKE A MAIN LINE TAP INTO AN EXISTING WATER MAIN. HOWEVER, IT IS THE RESPONSIBILITY OF THE CUSTOMER TO CORRECTLY SIZE BOTH THE SERVICE TAP AND SERVICE LINE. ALSO, IT IS UP TO THE UTILITY CONTRACTOR REQUESTING THE TAP TO PROVIDE THE APPROPRIATE TAPPING SLEEVE, IMPLEMENT APPROPRIATE MAINTENANCE OF TRAFFIC PLAN (IF REQUIRED), DO ALL EXCAVATING (IN ACCORDANCE WITH THE FLORIDA TRENCH SAFETY ACT), TO INSTALL THE PIPING, VALVE AND METER BOXES OR METER VAULTS, AND TO COMPLETE ALL BACKFILLING AND GRADE RESTORATION. THE UTILITY CONTRACTOR IS TO CLEARLY MARK THE MAIN AT THE LOCATION OF THE DESIRED TAP. THE UTILITY CONTRACTOR MUST BE PRESENT AT THE TIME OF THE TAP. THERE WILL BE A FEE FOR THIS MAIN LINE TAPPING SERVICE.
22. MAINTENANCE OF ALL WATER, SEWER AND STORMWATER FACILITIES WITHIN THE PROPERTY LINES WILL BE THE RESPONSIBILITY OF THE LAND OWNER OR CUSTOMER. THE CITY OF VALPARAISO WILL MAINTAIN THE WATER METERS WITHIN THE METER BOXES.
23. COORDINATE RELOCATION OF UTILITIES AND WORK WITHIN RIGHT OF WAY WITH AFFECTED UTILITY COMPANIES AND CITY OF VALPARAISO WATER AND SEWER DEPARTMENTS.
24. COORDINATE AND RECEIVE PLAN APPROVAL FROM CITY OF VALPARAISO PRIOR TO ANY ROAD CLOSURE OR PROPOSED MAINTENANCE OF TRAFFIC.
25. IT IS THE RESPONSIBILITY OF THE UTILITY CONTRACTOR/DEVELOPER TO MOVE ANY UTILITY SERVICES, STRUCTURES OR STUB OUTS THAT CONFLICT WITH DRIVEWAYS OR SIDEWALKS, AND/OR DOES NOT ADHERE TO THE SEPARATION REQUIREMENT AS STATED.

NOT RELEASED FOR CONSTRUCTION

JENKINS ENGINEERING, INC.
GCT ENGINEERING

JENKINS ENGINEERING, INC.
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DESTIN, FLORIDA 32541
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JEICIVIL.COM
C.O.A. NO. 9927 - L.B. NO. 8081

MATTHEW N. JENKINS, P.E.
PL REGISTRATION NO. 57642

REV	DATE	DESCRIPTION	BY
1	05-05-15	REVISED PER CITY OF VALPARAISO COMMENTS (JHT)	JHT

VALPARAISO REALTY COMPANY

JASMINE NORTH SUBDIVISION
CITY OF VALPARAISO, FLORIDA

UTILITY PLAN

JOB: E15020
DATE: 04-06-2016
DESIGNED: MS/MZ
DRAWN: MS

BAR IS ONE INCH ON ORIGINAL
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DRAWING NUMBER
5 OF 13

SHEET NUMBER
C04

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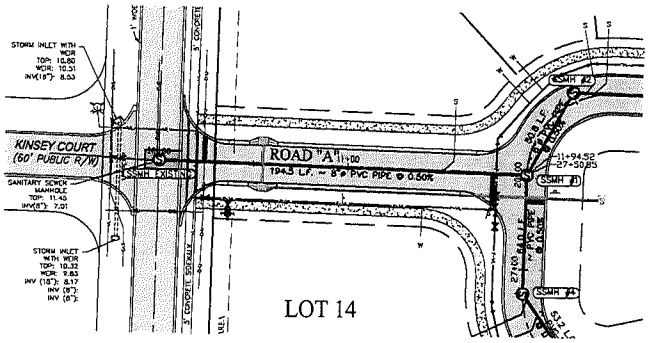
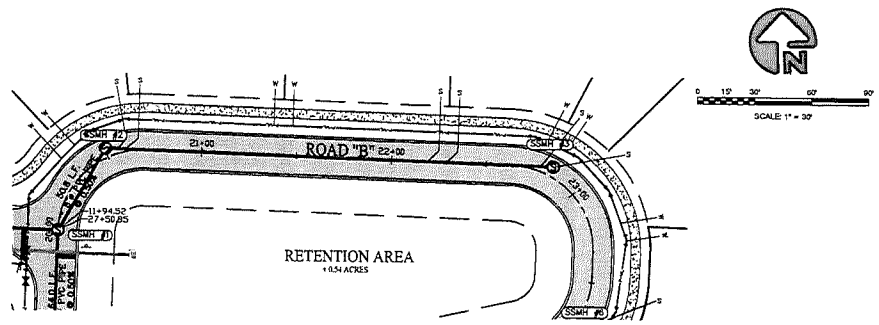
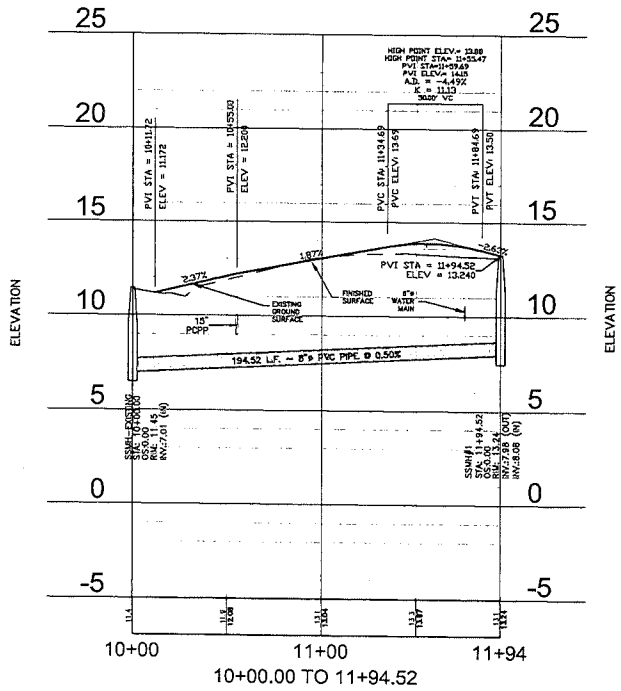
Last plotted by: Marek Scotka Plot Style: GCT2011.ctb Plot Scale: 1:2.59 Plot Date: 5/4/2016 9:36 AM Plotter used: None



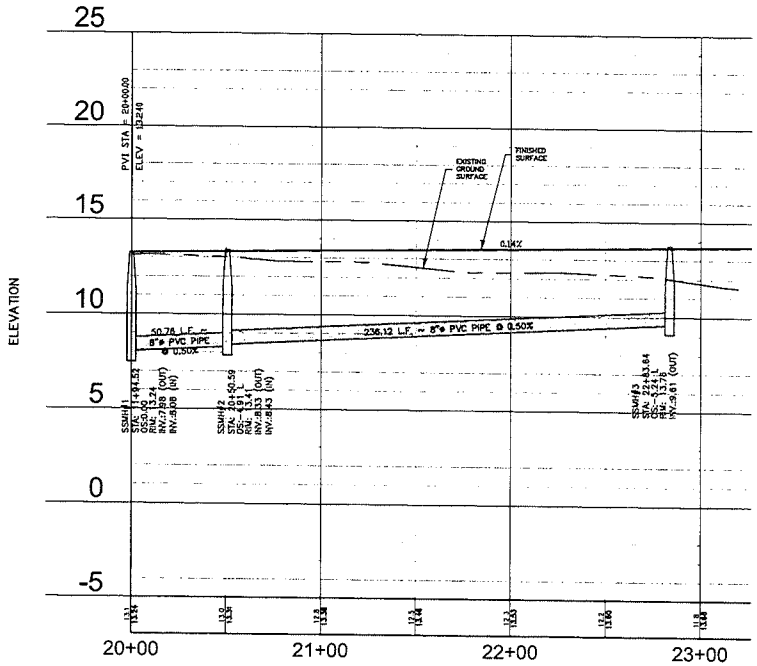
PLAN
ROAD "A"



PLAN
ROAD "B"



PROFILE
ROAD "A"



PROFILE
ROAD "B"

(POSTED/DESIGN SPEED 10 MPH)

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JENKINS ENGINEERING, INC.
GCT ENGINEERING

JENKINS ENGINEERING, INC.
1204 AIRPORT ROAD, SUITE 123
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PHONE 850.837.7446
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121 HART STREET
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JEICIVIL.COM
C.O.A. NO. 9927 - L.B. NO. 8081

MATTHEW H. ZINKE
CERTIFICATE No. 57642
STATE OF FLORIDA
PROFESSIONAL ENGINEER

MATTHEW H. ZINKE, P.E.
FL REGISTRATION NO. 57642

REV	DATE	DESCRIPTION	BY
1	05-05-16	REVISED PER CITY OF VALPARAISO COMMENTS DATED 04-11-16	
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VALPARAISO REALTY COMPANY

JASMINE NORTH SUBDIVISION
CITY OF VALPARAISO, FLORIDA

PLAN & PROFILE (ROAD "A & B")

NOT VALID UNLESS BEARING ENGINEER'S ORIGINAL SIGNATURE

JOB: E15020
DATE: 04-06-2016
DESIGNED: MS/MZ
DRAWN: MS

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0 1"
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6 OF 13

SHEET NUMBER
C05

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Last plotted by: Marek Scotka Plot Style: GCT2011.ctb Plot Scale: 1:2.58 Plot Date: 5/4/2016 9:41 AM Plotter used: None

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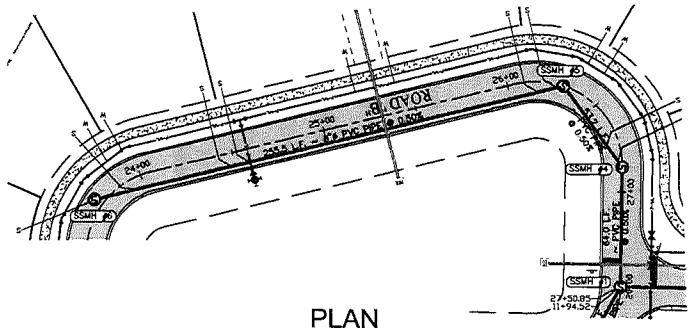
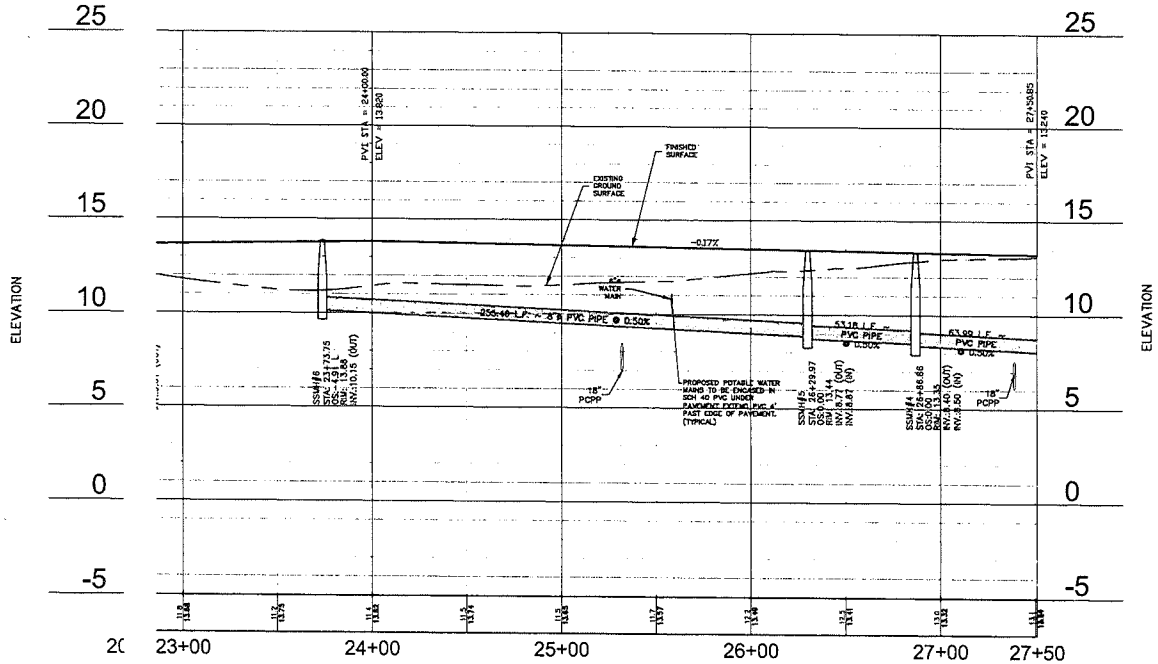


PLAN
ROAD "B"



PROFILE
ROAD "B"
(POSTED/DESIGN SPEED 10 MPH)

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GCT ENGINEERING

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JENKINS ENGINEERING, INC.
1234 AIRPORT ROAD, SUITE 125
DESTIN, FLORIDA 32541
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121 HART STREET
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C.O.A. NO. 9927 - L.B. NO. 8081

MATTHEW H. ZINKE
CERTIFICATE No. 57642
STATE OF FLORIDA
PROFESSIONAL ENGINEER

MATTHEW H. ZINKE, P.E.
FL REGISTRATION NO. 57642

REV	DATE	DESCRIPTION	BY
1	05-05-16	REVISED PER CITY OF VALPARAISO COMMENTS DATED 5-17-16	
2			
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VALPARAISO REALTY COMPANY

JASMINE NORTH SUBDIVISION
CITY OF VALPARAISO, FLORIDA

PLAN & PROFILE (ROAD "B")
NOT VALID UNLESS BEARING ENGINEERS ORIGINAL SIGNATURE

JOB: E15020
DATE: 04-06-2016
DESIGNED: MS/MZ
DRAWN: MS

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0 1"
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DRAWING NUMBER
7 OF 13

SHEET NUMBER
C06

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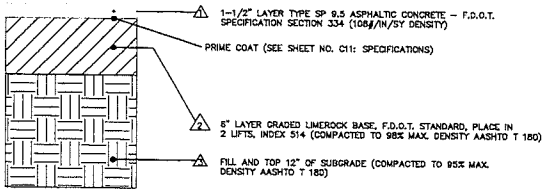
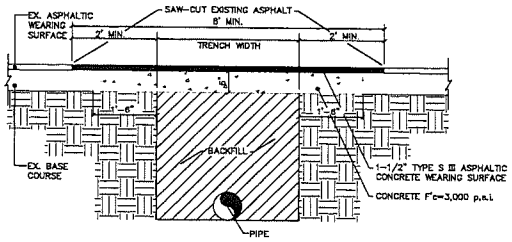
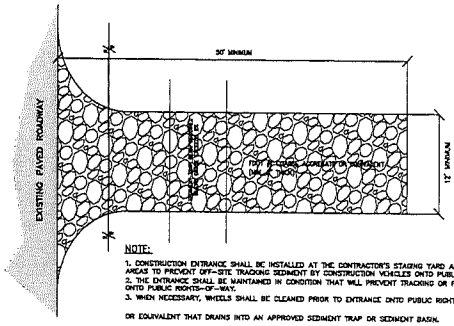


1 C07 **ASPHALTIC CONCRETE PAVEMENT SECTION**
N.T.S.



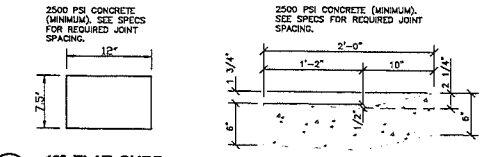
2 C07 **PAVEMENT PATCH DETAIL**
SCALE: 3/4" = 1'-0"



NOTE:
1. CONSTRUCTION ENTRANCE SHALL BE INSTALLED AT THE CONTRACTOR'S STAGING YARD AND/OR STOCKPILE AREAS TO PREVENT OFF-SITE TRACKING SEDIMENT BY CONSTRUCTION VEHICLES ONTO PUBLIC ROADS.
2. THE ENTRANCE SHALL BE MAINTAINED IN CONDITION THAT WILL PREVENT TRACKING OR FLOWING SEDIMENT ONTO PUBLIC RIGHTS-OF-WAY.
3. WHEN NECESSARY, WHEELS SHALL BE CLEANED PRIOR TO ENTRANCE ONTO PUBLIC RIGHT-OF-WAY.

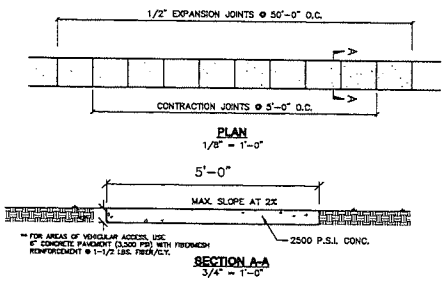
OR EQUIVALENT THAT DRAINS INTO AN APPROVED SEDIMENT TRAP OR SEDIMENT BASIN.

3 C07 **STABILIZED CONSTRUCTION ENTRANCE**
N.T.S.

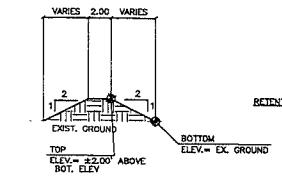


4 C07 **12" FLAT CURB**
SCALE: 1-1/2" = 1'-0"

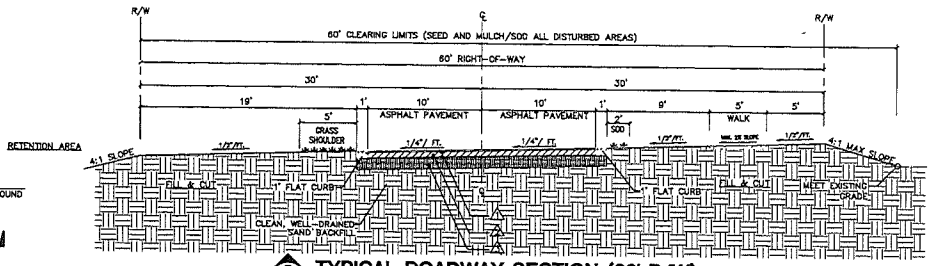
5 C07 **24" VALLEY GUTTER**
SCALE: 1-1/2" = 1'-0"



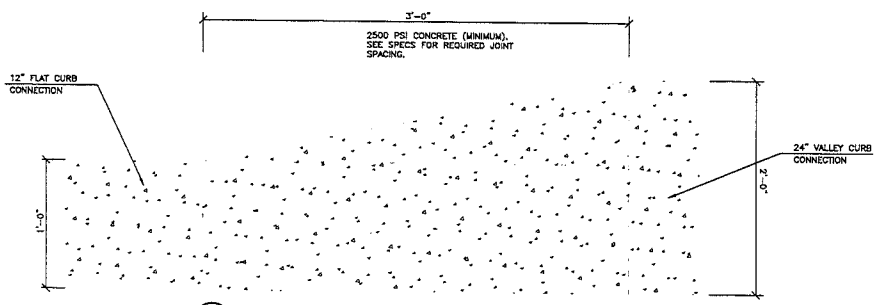
6 C07 **CONCRETE SIDEWALK DETAIL**
SCALE: AS SHOWN



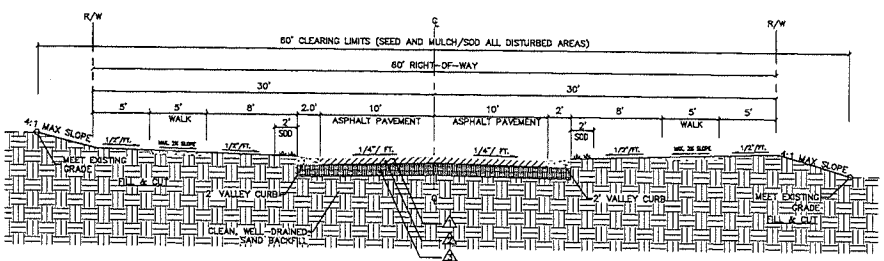
10 C07 **SEDIMENT TRAP BERM SECTION**
NOT TO SCALE



7 C07 **TYPICAL ROADWAY SECTION (60' R/W)**
SCALE: 1" = 5'-0"



8 C07 **3' TRANSITION FROM 12" FLAT CURB TO 24" VALLEY CURB**
SCALE: 1-1/2" = 1'-0"



9 C07 **ENTRANCE ROADWAY SECTION (60' R/W)**
SCALE: 1" = 5'-0"

JENKINS ENGINEERING, INC.
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MATTHEW H. ZIMKE CERTIFICATE No. 57642 STATE OF FLORIDA PROFESSIONAL ENGINEER

MATTHEW H. ZIMKE, P.E.
FL REGISTRATION NO. 57642

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1	05-05-16	REVISED PER CITY OF VALPARAISO COMMENTS DATED 04-11-16	
2			
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VALPARAISO REALTY COMPANY

JASMINE NORTH SUBDIVISION
CITY OF VALPARAISO, FLORIDA

MISCELLANEOUS DETAILS

NOT VALID UNTIL REVIEWED BY THE CITY ENGINEER & SIGNED & SEALED BY THE ENGINEER OF RECORD

JOB: E15020
DATE: 04-06-2016
DESIGNED: MS/MZ
DRAWN: MS

BAR IS ONE INCH ON ORIGINAL
0 1"
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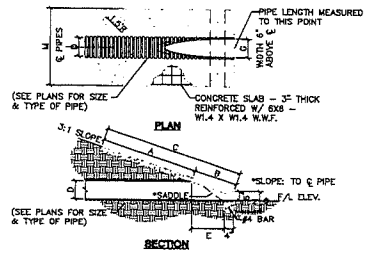
6 OF 13

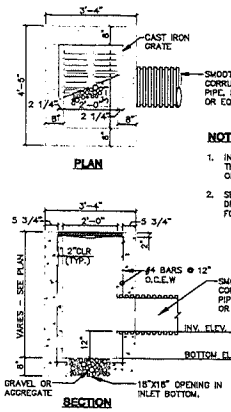
SHEET NUMBER
C07

File: J:\Eng-proj\2015\E15020 VRO Jasmine Ave North SD\dwg\E15020 Jasmine North-8A.DWG Last Saves: 5/4/2016 9:40 AM Last saved by: Mscotka
Last plotted by: Marek Scotka Plot Style: GCT2011.ctb Plot Scale: 1:2.58 Plot Date: 5/4/2016 9:41 AM Plotter used: None

DIMENSIONS							
D	A	B	C	G	M	N	E
15"	2.27'	4.09'	6.36'	1.22'	4.83'	1.19'	4.03'
18"	2.36'	5.12'	7.48'	1.41'	4.92'	1.21'	5.03'
24"	2.53'	7.18'	9.71'	1.73'	5.50'	1.25'	7.03'



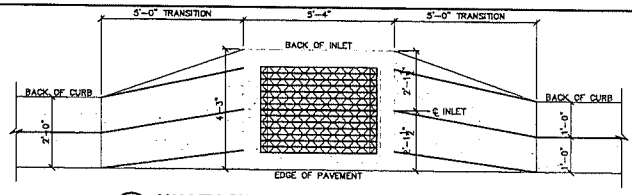
1 C08 MITERED END SECTION
SCALE: 1/2" = 1'-0"



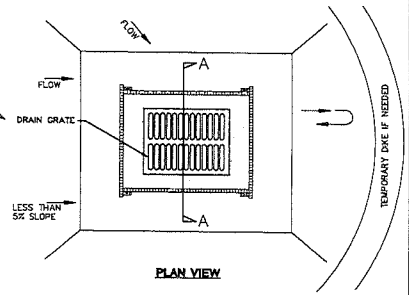
NOTES:

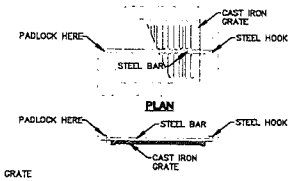
1. INLET BOTTOM 12" LOWER THAN PIPE INVERT SHOWN ON PLAN.
2. SEE F.D.O.T. STANDARD INDEX DRAWING NO.'s 201 & 232 FOR ADDITIONAL DETAILS.

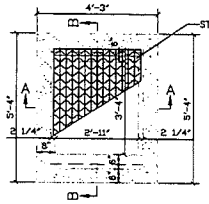
2 C08 OPEN BOTTOM TYPE "C" INLET
SCALE: 1/2" = 1'-0"



3 C08 VALLEY GUTTER-GUTTER INLET TRANSITION
SCALE: 1/2" = 1'-0"



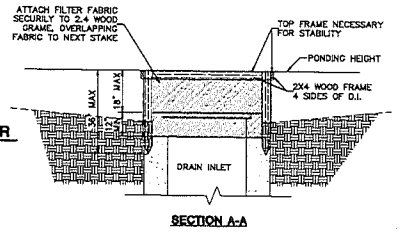




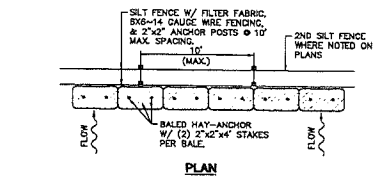
9 C08 BUBBLE-UP INLET LIFT OFF COVER
SCALE: 1/2" = 1'-0"

NOTES:

1. SEE F.D.O.T. STANDARD INDEX DRAWING NO.'s 201 & 221 FOR ADDITIONAL DETAILS.
2. INLET TOP SHALL BE SET TO MATCH SLOPE ALONG CENTERLINE ROADWAY PROFILE.

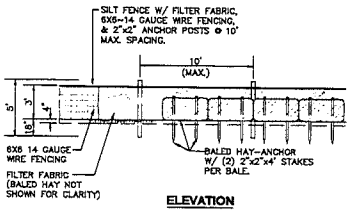


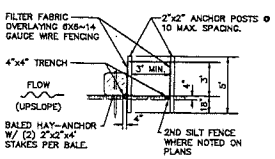
7 C08 INLET PROTECTION DETAIL
N.T.S.



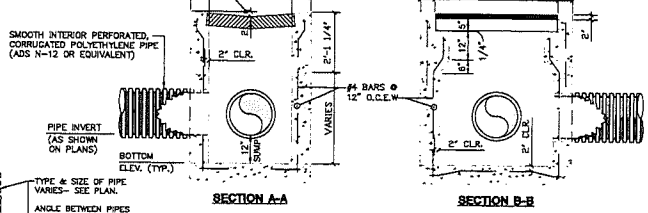
CONSTRUCTION NOTE:

INSTALL 2"X2" WOOD POSTS ALONG BARRIER LINE (10' MAX. SPACING, 18" MIN. EMBEDMENT). EXCAVATE 4" WIDE BY 4" DEEP TRENCH ALONG POST LINE, UPSLOPE OF BARRIER. FASTEN WIRE MESH FENCING TO 2"X2" POSTS WITH HEAVY DUTY WIRE STAPLES AT LEAST 1 INCH LONG, TIE WIRES OR HOG RINGS. STAPLE OR WIRE FILTER FABRIC TO UPSLOPE SIDE OF SUPPORT FENCING. EXTEND 8" OF FABRIC AND FENCING INTO TRENCH AND BACKFILL. STAKE BALED HAY UPSLOPE OF SILT FENCE ASSEMBLY.

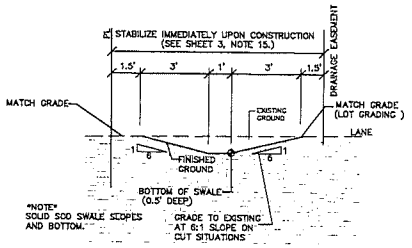




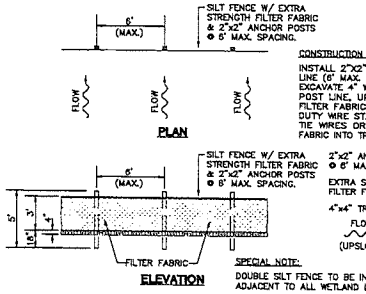
4 C08 TYPICAL EROSION CONTROL DETAILS
SCALE: 1" = 5'-0"



6 C08 TYPE "V" GUTTER INLET
SCALE: 1/2" = 1'-0"



5 C08 CONVEYANCE SWALE
NOT TO SCALE



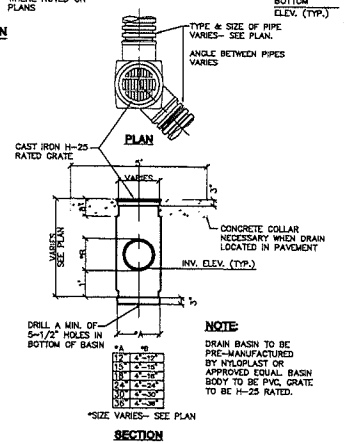
CONSTRUCTION NOTE:

INSTALL 2"X2" WOOD POSTS ALONG BARRIER LINE (6' MAX. SPACING, 18" MIN. EMBEDMENT). EXCAVATE 4" WIDE BY 4" DEEP TRENCH ALONG POST LINE, UPSLOPE OF BARRIER. FASTEN FILTER FABRIC TO 2"X2" POSTS WITH HEAVY DUTY WIRE STAPLES AT LEAST 1 INCH LONG, TIE WIRES OR HOG RINGS. EXTEND 8" OF FABRIC INTO TRENCH AND BACKFILL.

SPECIAL NOTE:

DOUBLE SILT FENCE TO BE INSTALLED ADJACENT TO ALL WETLAND LINES

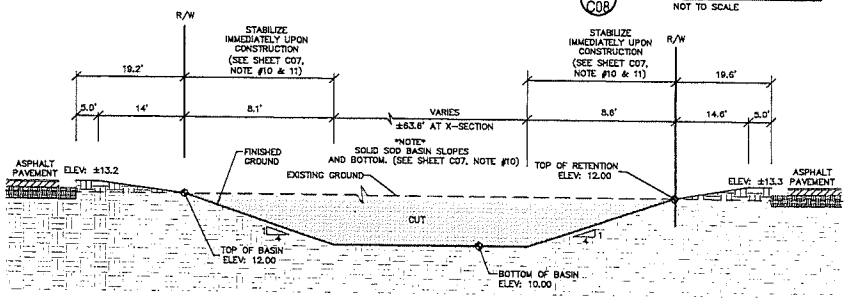
8 C08 SILT FENCE DETAIL
SCALE: 1" = 5'-0"



NOTE:

DRAIN BASIN TO BE PRE-MANUFACTURED BY NYLOPLAST OR APPROVED EQUAL. BASIN BODY TO BE PVC, GRATE TO BE H-25 RATED.

11 C08 NYLOPLAST INLET
SCALE: 1/2" = 1'-0"



10 C08 RETENTION BASIN
NOT TO SCALE

NOT RELEASED FOR CONSTRUCTION

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GCT ENGINEERING

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FAX: 850.678.2350
JEIGIVIL.COM
C.O.A. NO. 9927 - L.B. NO. 8081
MATTHEW H. ZIMMER CERTIFICATE No. 57642 STATE OF FLORIDA PROFESSIONAL ENGINEER
MATTHEW H. ZIMMER, P.E.
FL REGISTRATION NO. 57642

REV	DATE	DESCRIPTION	BY
1			
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8			

VALPARAISO REALTY COMPANY

JASMINE NORTH SUBDIVISION
CITY OF VALPARAISO, FLORIDA

MISCELLANEOUS DETAILS

K:\01JOBS\E15020\DWG\E15020 - JASMINE NORTH - MISC DETAILS.DWG

JOB: E15020
DATE: 04-06-2016
DESIGNED: MS/MZ
DRAWN: MS

BAR IS ONE INCH ON ORIGINAL
0 1"
IF NOT ONE INCH ON THIS SHEET ADJUST SCALES ACCORDINGLY

DRAWING NUMBER 9 OF 13

SHEET NUMBER C08

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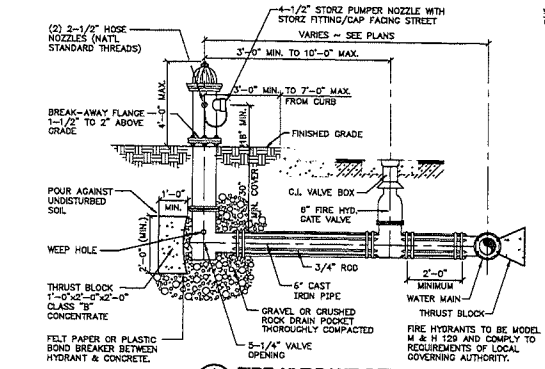
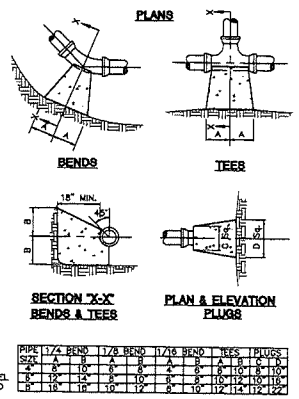
File: J:\Eng-pro\2015\E15020 VRC Jasmine Ave North SD\dwg\E15020 Jasmine North-8A.DWG Last Save: 5/4/2016 9:40 AM Last saved by: Macotka
Last plotted by: Mark Scotka Plot Style: GCT2011.ctb Plot Scale: 1:2.58 Plot Date: 5/4/2016 9:41 AM Plotter used: None



1 C09 FIRE HYDRANT DETAIL

SCALE : 1/2" = 1'-0"

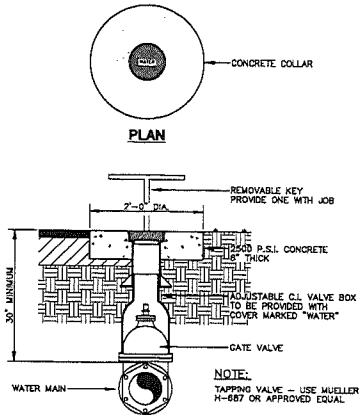
NOTE: PLACE FELT PAPER OR PLASTIC BOND BREAKER BETWEEN BLOCK AND CONCRETE.



PIPE SIZE	1/4 BEND A	1/4 BEND B	1/8 BEND A	1/8 BEND B	1/16 BEND A	1/16 BEND B	TEES A	TEES B	PLUGS A	PLUGS B
4"	8"	10"	6"	8"	4"	6"	6"	10"	8"	10"
6"	12"	14"	8"	10"	6"	8"	8"	10"	12"	18"
8"	16"	18"	10"	12"	8"	10"	12"	14"	12"	22"

2 C09 THRUST BLOCK DETAILS

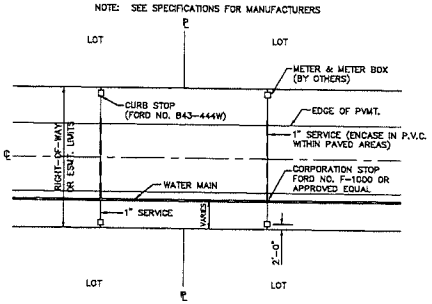
N.T.S.



3 C09 GATE VALVE AND VALVE BOX DETAIL

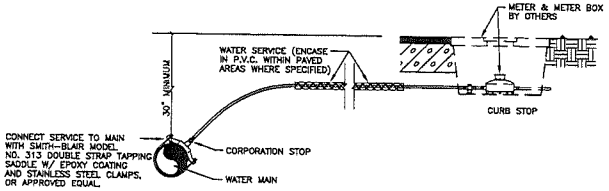
N.T.S.

NOTE: SEE SPECIFICATIONS FOR MANUFACTURERS



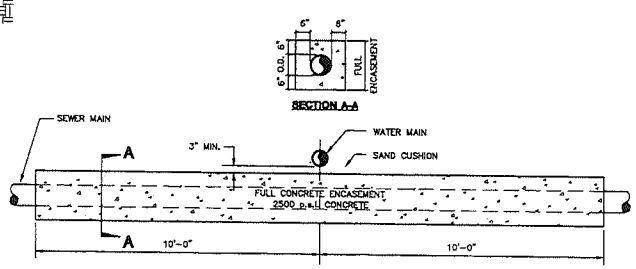
4 C09 SINGLE WATER SERVICE CONNECTION PLAN

SCALE: 1" = 20'



5 C09 WATER SERVICE CONNECTION DETAIL

N.T.S.



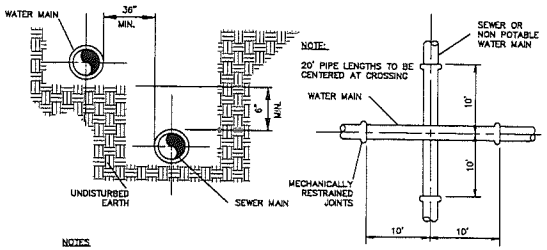
NOTE: FOR POTABLE WATER LINE CROSSING, 1-1/2" DIAMETER AND LARGER, WATER MAIN SHALL BE LOCATED ABOVE OR BELOW ENCASEMENT AS SHOWN ON PLANS OR AS DETERMINED IN THE FIELD. USE ENCASEMENT WHERE VERTICAL CLEARANCE BETWEEN WATER MAIN AND SEWER IS LESS THAN 18".

FOR ALTERNATE:

CAST IRON OR DUCTILE IRON PIPES MAY BE USED IN LIEU OF CONCRETE ENCASEMENT AT WATER LINE CROSSING, WITH A LENGTH OF PIPE CENTERED AT THE POINT OF CROSSING, SO AS TO LOCATE JOINTS AT A MAXIMUM DISTANCE FROM THE WATER LINE.

6 C09 ENCASEMENT AT WATER LINE DETAIL

SCALE: 1/2" = 1'-0"



NOTES:

1. WHERE A 6 FOOT PARALLEL SEPARATION CANNOT BE MAINTAINED BETWEEN A WATER MAIN AND A SEWER MAIN OR NONPOTABLE WATER MAIN, THEN THE WATER MAIN SHALL BE LAID IN A SEPARATE TRENCH OR ON AN UNDISTURBED EARTH SHELF WITH 3 FOOT MINIMUM HORIZONTAL SEPARATION (MEASURED OUTSIDE OF PIPE) AND 8 INCHES MINIMUM (12 INCHES PREFERRED) ABOVE THE TOP OF THE SEWER MAIN OR NONPOTABLE WATER MAIN.

2. WHERE A 12 INCH VERTICAL SEPARATION DISTANCE CANNOT BE MAINTAINED BETWEEN A WATER MAIN AND A SEWER MAIN OR NONPOTABLE WATER MAIN CROSSING THEN THE WATER MAIN SHALL PASS OVER THE SEWER MAIN OR NONPOTABLE WATER MAIN WITH A MINIMUM VERTICAL CLEARANCE OF 6 INCHES. THE SEWER OR NONPOTABLE WATER MAIN SHALL BE 20 FEET OF 6" CONCRETE ENCASEMENT CENTERED ON THE POINT OF CROSSING AND THE WATER MAIN SHALL BE 20 FEET OF 6" CONCRETE ENCASEMENT CENTERED ON THE POINT OF CROSSING. CONCRETE ENCASEMENT SHALL BE POURED SEPERATELY FOR EACH PIPE.

7 C09 WATER/SEWER SEPARATION DETAIL

NOT TO SCALE

GENERAL NOTES:

1. The contractor shall be responsible for obtaining bacteriological examinations on all new water mains (as required by and to the satisfaction of) the Department of Health and Rehabilitative Services. These requirements necessitate a satisfactory sample at each sample location for two consecutive days. If the examination is by persons other than the local health department, then the contractor shall be responsible for delivering the laboratory reports to the engineer for submittal to the State Department of Environmental Protection (D.E.P.).
2. Installation of water mains and services shall comply with all governing authorities.
3. The contractor shall verify all field dimensions and report any discrepancies prior to commencing work.
4. All pipes shall be P.V.C. unless otherwise noted or required. All mains to be marked with No. 14 insulated copper tracer wire to facilitate their locations.
5. The contractor shall provide a minimum of 36" of cover over water mains within paved and unpaved areas.

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JEICIVIL.COM
C.O.A. NO. 9927 - L.B. NO. 8081

MATTHEW M. ZIMAS, P.E.
FL REGISTRATION NO. 57642

REV	DATE	DESCRIPTION	BY
1			
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VALPARAISO REALTY COMPANY

JASMINE NORTH SUBDIVISION
CITY OF VALPARAISO, FLORIDA

POTABLE WATER DETAILS

NOT VALID UNLESS SEALED & SIGNED BY A FLORIDA LICENSED ENGINEER

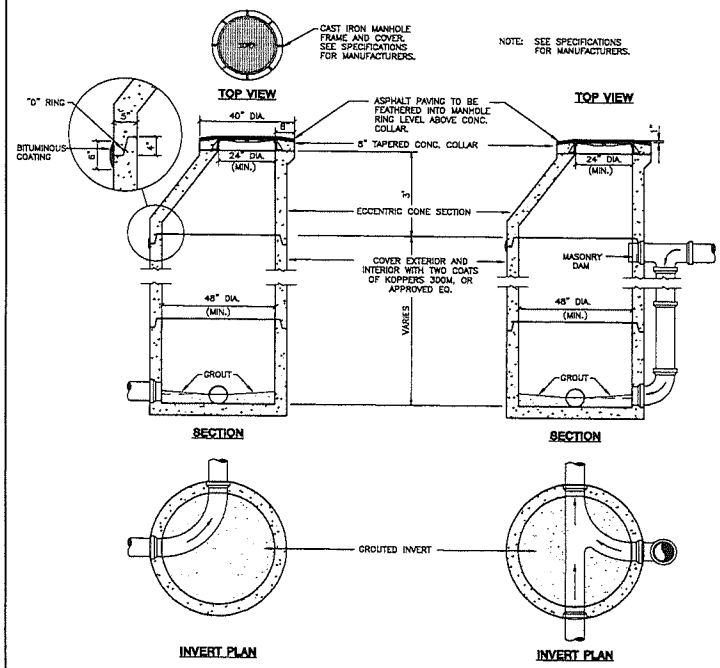
JOB: E15020
DATE: 04-06-2016
DESIGNED: MS/MZ
DRAWN: MS

BAR IS ONE INCH ON ORIGINAL
IF NOT ONE INCH ON THIS SHEET ADJUST SCALES ACCORDINGLY

DRAWING NUMBER
10 OF 13

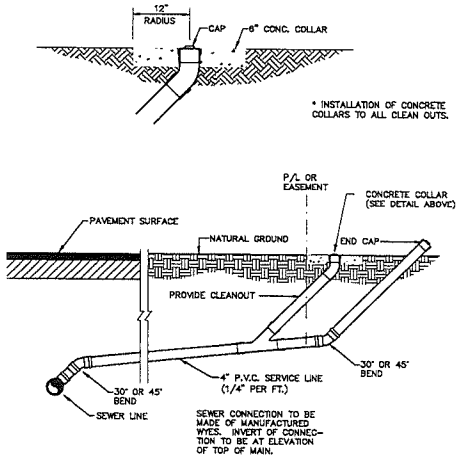
SHEET NUMBER
C09

File: J:\Eng-proj\2015\E15020 VRG Jasmine Ave North SD\dwg\E15020 Jasmine North-8A.DWG Last Save: 5/4/2016 9:40 AM Last saved by: Mscotka
Last plotted by: Marek Scotka Plot Style: GCT2011.ctb Plot Scale: 1:2.58 Plot Date: 5/4/2016 8:41 AM Plotter used: None

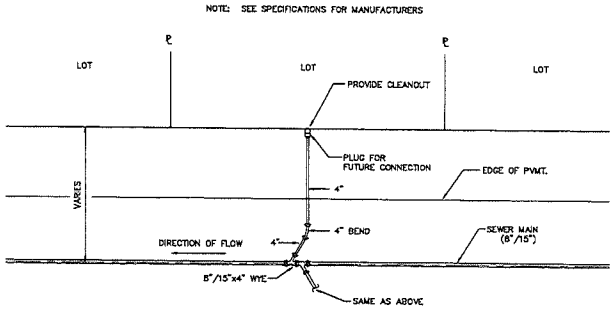


1 C10 STANDARD MANHOLE DETAIL
SCALE: 1/2" = 1'-0"

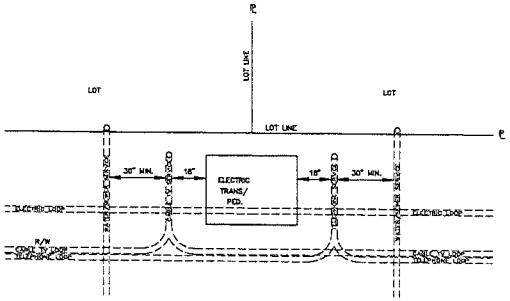
2 C10 DROP MANHOLE DETAIL
SCALE: 1/2" = 1'-0"



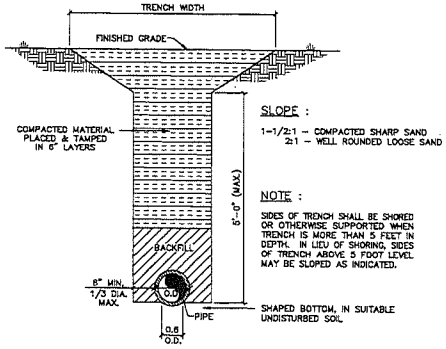
3 C10 SEWER SERVICE DETAIL
SCALE: 1/2" = 1'-0"



4 C10 SINGLE SANITARY SEWER SERVICE CONNECTION PLAN
SCALE: 1" = 10'



7 C10 TYPICAL LOT CORNER UTILITY DETAIL
SCALE: 1" = 10'



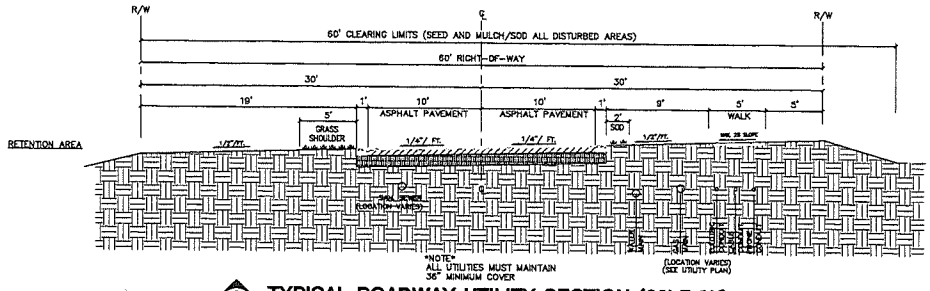
SLOPE :

1-1/2:1 – COMPACTED SHARP SAND
2:1 – WELL ROUNDED LOOSE SAND

NOTE :

SIDES OF TRENCH SHALL BE SHORED OR OTHERWISE SUPPORTED WHEN TRENCH IS MORE THAN 5 FEET IN DEPTH. IN LIEU OF SHORING, SIDES OF TRENCH ABOVE 5 FOOT LEVEL MAY BE SLOPED AS INDICATED.

5 C10 PIPE BEDDING DETAIL
SCALE: 3/4" = 1'-0"



6 C10 TYPICAL ROADWAY UTILITY SECTION (60' R/W)
SCALE: 1" = 5'-0"

GENERAL NOTES :

1. ALL CONCRETE TO HAVE A 28 DAY COMPRESSIVE STRENGTH OF 4000 P.S.I. UNLESS OTHERWISE NOTED.
2. PROVIDE MANHOLE STEPS FOR MANHOLES WITH DEPTHS GREATER THAN 42 INCHES.
3. GROUT ALL PIPE ENTRIES AND MANHOLE INVERTS.
4. INVERT GROUTING TO BE UNIFORM AND SMOOTH – SLOPED TO CENTER-LINE OF PIPE.

NOT RELEASED FOR CONSTRUCTION

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MATTHEW H. JONES CERTIFICATE No. 57642 STATE OF FLORIDA PROFESSIONAL ENGINEER

MATTHEW H. JONES, P.E.
Digitally signed by Matthew H. Jones
DN: c=US, o=Jenkins Engineering, Inc., cn=Matthew H. Jones, email=matt@jeicivil.com
Date: 2016.05.18 09:40:00 -05'00'

REV	DATE	DESCRIPTION	BY
1	04-05-16	REVISED PER CITY OF VALPARAISO COMMENTS DATED 3-17-16	
2			
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VALPARAISO REALTY COMPANY

JASMINE NORTH SUBDIVISION
CITY OF VALPARAISO, FLORIDA

SANITARY SEWER DETAILS

H:\01 MARKETING\SIGNING PROJECTS\SIGNATURE

JOB: E15020
DATE: 04-06-2016
DESIGNED: MS/MZ
DRAWN: MS

BAR IS ONE INCH ON ORIGINAL
0 1"
IF NOT ONE INCH ON THIS SHEET ADJUST SCALES ACCORDINGLY

DRAWING NUMBER
11 OF 13

SHEET NUMBER
C10

File: J:\Eng-pro\2015\E15020 VRC Jasmine Ave North SD\dwg\E15020 Jasmine North-SA.DWG Last Save: 5/4/2016 9:40 AM Last saved by: Mscotka
Last plotted by: Marek Scotka Plot Style: GCT2011.ctb Plot Scale: 1:2.59 Plot Date: 5/4/2016 9:42 AM Plotter used: None

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SECTION 1
CLEARING AND GRUBBING

1-1 Description

The work specified in this Section consists of clearing and grubbing within the project limits. Included in the work under this Section is the removal and disposal of existing pavement, as well as all protruding objects such as trees, stumps, roots, etc., necessary to prepare the area for the proposed construction, and removal and disposal of all product and debris which are not required to be salvaged or not required to complete the construction

1-2 Work Included

Clearing and grubbing shall consist of the complete removal and disposal of all timber, brush, stumps, roots, rubbish, and debris and all other construction resting on or protruding through the surface of the existing ground and the surface of excavated areas, and all other structures and obstructions necessary to be removed and for which the removal thereof is not specified to be done under other items of the contract. Unless otherwise shown in the plans, clearing and grubbing shall be done in the following areas.

1. All areas where excavation is to be done including borrow pits, lateral ditches, right-of-way ditches, etc
2. All areas where embankments will be constructed
3. All areas where structures will be constructed
4. Any other areas specifically called for on the plans to be cleared and grubbed

1-3 Depths of Removal of Roots, Stumps, and Other Debris

In all areas where excavation is to be done and where the excavated material is to be used in the construction of embankments, or roadway base, also in all areas where roadway embankment will be constructed, roots, and other debris shall be removed to a depth of at least one foot below ground surface. The surface shall then be plowed to a depth of at least six inches and all roots thereby exposed shall be removed to a depth of at least one foot. All stumps within the construction limits shall be removed and disposed of by the Contractor. Within all other areas where clearing and grubbing is to be done, roots and other debris projecting through or appearing on the surface of the original ground shall be removed to a depth of one foot below the surface; no plowing and harrowing will be required in these areas.

1-4 Disposal of Materials

Timber, stumps, brush, roots, and other objectionable material resulting from the clearing and grubbing shall be disposed of by the Contractor in locations and by methods approved by Okaloosa County.

END OF SECTION

SECTION 2
EXCAVATION AND EMBANKMENT

2-1 Description

The work specified in this Section consists of the excavation and embankment required for the roadway and ditches; the excavation and backfilling of pipe and utility trenches, and includes the preparation of subgrades, the construction of embankments, and other utilization or satisfactory disposal of the materials excavated, and the compaction and dressing of excavated areas and embankments.

2-2 Excavation

2-2.1 Classification of Excavation

All excavation is classified as regular excavation. Regular excavation shall consist of the excavation and the utilization or satisfactory disposal of all materials necessary for the construction of the roadway and side ditches, and the installation of pipe and utility trenches.

2-2.2 Excavation for Pipework

All excavation shall be made along straight lines by open cut unless otherwise authorized by the Engineer or otherwise shown on the plans. Holes for pipe bells shall be hand-excavated to insure that the pipe rests upon the bottom of the trench for its entire length. If the bottom of the excavation is found to consist of rock, or any material that cannot be excavated to give a uniform bearing pressure, the material shall be removed to a depth at least six inches below established bottom grade and backfilled to grade with thoroughly compacted sand at the Contractor's expense. Any excavation carried below the depths indicated, without specific direction from the Engineer, shall be backfilled in the same manner. Trench width measured at one foot above the top of pipe for all pipe other than that approved for assembly prior to installation in the trench shall be:

1. The outside diameter of the pipe plus 16 inches for pipe up to and including 30 inches inside diameter.
2. The outside diameter of the pipe plus 24 inches for pipe greater than 30 inches inside diameter
3. Where sheeting or shoring is used the allowable width shall be measured between the inside face of the sheeting or shoring.

For all pressure line piping approved for complete assembly prior to installation in the trench, the minimum trench width shall be the outside diameter of the coupling or joint plus two inches. Excavated material to be used for backfill shall be neatly deposited at the sides of the trenches where space is available. Where stockpiling of the excavated material is required, the Contractor shall obtain the sites to be used and maintain operations so as to provide for natural drainage and not present an unsightly appearance. Rock, shell, or other base materials for roads shall be carefully selected and kept separate. Grade and line stakes shall be protected. No excavated material shall be placed on private property other than the Owner's.

2-2.3 Dewatering for Pipework

Dewatering, if required, shall be continued during construction to keep the ground water below the level of the back-fill at all times until the backfill is completed. Water setting may be approved or required, and shall consist of continuing the well points in service and applying water as directed to the excavation during backfill

2-2.4 Shoring

When necessary to protect workmen, banks, adjacent paving, structures and utilities, excavations shall be shored and braced by members of suitable size and arrangement. Shoring, bracing and sheeting shall be removed as excavations are backfilled, in a manner to prevent injurious caving. Where directed by the Engineer, the sheeting shall be left in place in the backfill with proper bracing to provide lateral support.

2-3 Disposal of Unsuitable Materials

If not otherwise designated in the plans, unsuitable materials shall be disposed of by the Contractor in areas provided by him, to the satisfaction of the Engineer and Okaloosa County

2-4 Disposal of Excess Useable Materials

Excess useable materials shall be disposed of by the Contractor in areas provided by the Owner, and to the satisfaction of the Owner. The determination whether material is classified as useable or unsuitable shall be up to the judgment of the Engineer.

2-5 Materials for Embankment

All suitable material resulting from the excavation shall be used as far as practical in the construction of the roadway. Embankment shall be constructed of material containing no muck, stumps, roots, brush, vegetable matter, rubbish or other material that will not compact into a suitable and enduring roadbed, and material designated as undesirable shall be removed as provided above. Material placed over the areas of the project which are to be grassed, seeded and mulched, or sodded shall be suitable for plant growth and free from appreciable quantities of hard clods, stiff clay, hardpan, gravel, brush, roots, refuse, or other deleterious materials and shall be of reasonably uniform quality

2-6 Embankment Construction

Embankment shall be constructed true to lines, grades, and cross sections shown in the plans or as ordered by the Engineer within the tolerances specified herein.

2-7 Backfilling of Pipework

2-7.1 General

All fill and backfill shall be free from organic matter such as roots, stumps, trees, refuse, or other objectionable material. Except as specified otherwise, fill and backfill shall be placed in layers not more than 6 inches thick and each layer shall be compacted thoroughly and evenly. The moisture content of the fill material shall be such that proper compaction will be obtained. Backfill shall be not placed against concrete within seven days after it has been poured and only when directed by the Engineer

2-7.2 Backfilling Trenches

The initial backfill shall be carefully deposited on both sides of the pipe at the same time and thoroughly compacted around the barrel of the pipe until enough backfill has been placed to provide a cover of one foot above the bell of the pipe. The remainder of the trench shall be backfilled in well compacted one-foot layers, except for trenches excavated in roads and streets the backfill shall be placed and compacted to the density specified in Article 2-8.4, with approved mechanical tampers in six inch layers to the top of the trench. Water setting may be used where approved and shall be used where directed by the Engineer. The top material shall be used last and the surface of the trench restored to its original elevation. Under no conditions is construction debris to be included with the backfill. Excavated material consisting of muck, mud, clay or other unsuitable material may not be utilized in the backfill. Where sheeting is withdrawn, all cavities remaining in or adjoining the trench shall be solidly filled and thoroughly compacted. Where sheeting is to remain in place, all cavities behind it shall be backfilled in the same manner as specified for trench backfill. No sheeting that has been driven below the pipe invert may be removed. Before backfilling is completed all sheeting to remain shall be cut off at a line three feet below finish grade.

2-8 Compaction Requirements

2-8.1 Compaction of Embankments

Each layer of the material used in the formation of the embankments shall be compacted to a density of at least 95 percent of the maximum density as determined by AASHTO T 180. Each layer shall be uniformly compacted, using equipment which will achieve the required density, and as compactive operations progress, each layer shall be shaped and manipulated as necessary to assure uniform density throughout the embankment

2-8.2 Compaction of Subgrade

The subgrade in both cuts and fills shall be compacted to a density of at least 98 percent of the maximum density as determined by AASHTO T 180. It shall be the Contractor's responsibility to maintain the required density until the base or pavement, as applicable, is placed on the subgrade. Compaction tests shall be provided by the Contractor, at his expense, at intervals of no more than two hundred feet, staggered to the left, right, and on centerline to verify density. A copy of the density test results shall be provided to the Engineer for approval prior to beginning base operations.

2-8.3 Compaction of Grassed Areas

For the upper six-inch layer of areas to be grassed, no specific density will be required under this Section and compaction shall be only to the extent directed.

2-8.4 Compaction for Pipes, Culverts, Etc

The backfill for trenches shall be compacted to a density of at least 98 percent of the maximum density as determined by AASHTO T 180 and in accordance with other sections of these specifications. Embankment over and around pipes and culverts shall be thoroughly compacted in a manner which will not place undue stress on the structures

2-9 Final Dressing

As a final grading operation, the surface of the earth work shall be shaped to conform to the lines, grades, and cross sections shown in the plans, or as directed, within the tolerances specified below.

A tolerance of 0.3 foot above or below plan cross section will be allowed on the final earth work surface with the following exceptions

1. The surface of shoulders shall be shaped to within 0.1 foot of the plan cross section
2. Earth work shall be shaped to match adjacent pavement, curb, sidewalk, structures, etc
3. Ditch bottoms shall be shaped so that no water will be impounded.

When dressing area adjacent to pavement, care shall be exercised to avoid possible damage to such pavements

2-10 Inspection

The Contractor shall be responsible for notifying the County Engineer to arrange for inspections of the embankment and subgrade. The County Engineer shall be notified twenty-four (24) hours before the construction is ready for inspection

END OF SECTION

SECTION 3
GRADED AGGREGATE BASE COURSE

3-1 Description

The work specified in this Section consists of the construction of a base course composed of graded aggregate. The base shall be constructed on a prepared subgrade and in a single course, and shall be constructed in accordance with these specifications and in conformity with the lines, grades, thickness, and cross-section shown in the plans.

3-2 Materials

The material shall be a Group 1 graded aggregate from an FDOT approved source and meet the requirements of FDOT *Standard Specifications for Road and Bridge Construction 2007. Section 204 Graded Aggregate Base*. The Contractor is to provide laboratory results from a Florida registered Geotechnical Engineer indicating that material to be used meets or exceeds the above requirements

3-2.1 Stockpiling Materials

Aggregate shall be stockpiled on the cleared and leveled areas designated by the Engineer. The material shall be placed in such a manner so as to prevent segregation and allow for drainage of water.

3-2.2 Mixing of Materials

The coarse and fine aggregates shall be mixed in a stationary plant, or in a traveling plant or bucket loader on an approved paved working area, and delivered as one mixture containing the fine and coarse aggregates. The supplier shall make such adjustments in mixing procedures or in equipment as may be directed to minimize segregation or degradation, and to ensure a satisfactory base course meeting all requirements of this specification.

3-3 Equipment

The crushed aggregate shall be spread by mechanical rock spreaders, equipped with a device which strikes off the rock uniformly to the laying thickness and capable of producing an even distribution of the rock.

3-4 Spreading, Shaping, and Compacting

3-4.1 General

The crushed aggregate shall be spread uniformly as specified above. All segregated areas of fine or coarse aggregate shall be removed and replaced with properly graded aggregate. After the spreading is completed the entire surface shall be scarified and then shaped so as to produce the required grade and cross section after compaction.

3-4.2 Moisture Content

When the material does not have the proper moisture content to insure the required density, wetting or drying will be required. When water is added it shall be uniformly mixed-in by disking to the full depth of the course which is to be compacted. Wetting or drying operations shall involve manipulation, as a unit, of the entire width and depth of the course which is to be compacted

3-4.3 Density Requirements

The base material shall be compacted to a density of not less than 98 percent of maximum density as determined AASHTO T 180 as soon as proper conditions of moisture are obtained.

3-4.4 Density Tests

Compaction tests shall be provided by the Contractor, at his expense, at intervals of no more than two hundred feet, staggered to the left, right, and on centerline to verify density. A copy of the density tests shall be provided to the Engineer for approval prior to beginning paving operations.

During final compacting operations, if blading of any areas is necessary to obtain the true grade and cross section, the compacting operations for such areas shall be completed prior to making the density tests on the finished base.

3-5 Testing Surface

The finished surface of the base course shall be checked with a 15-foot straightedge laid parallel to the center line of the road. All irregularities greater than 1/4 inch shall be corrected by scarifying and removing or adding crushed aggregate as required, after which the entire area shall be recompacted as specified herein before.

3-6 Thickness Requirements

The thickness of the base shall be measured at intervals of not more than 200 feet. Measurements shall be taken at various points on the cross section, through holes not less than three inches in diameter. Where the compacted base is deficient by more than 1/2 inch from the required thickness, the Contractor shall correct such areas by scarifying and adding aggregate. The base shall be scarified and aggregate added for a distance of 100 feet in each direction from the edge of the deficient area. The affected area shall then be brought to the required state of compaction and to the required thickness and cross section. A copy of the base thickness testings results shall be provided to the Engineer for approval prior to beginning paving operations. Verification thickness testing shall also be performed by the County Engineer. The Contractor shall be responsible for notifying the County Engineer for base inspection and base thickness testing. The County Engineer shall be notified twenty-four (24) hours before the construction is ready for inspection.

3-7 Maintenance

The Contractor shall be responsible for assuring that the true crown and grade are maintained, with no rutting or other distortion, and that the base meets all other requirements of these specifications at the time of paving.

END OF SECTION

SECTION 4
ASPHALTIC CONCRETE PAVING

4-1 Description

The work specified in this Section consists of constructing an asphaltic concrete wearing surface to a uniform grade and cross-section. The work shall be in accordance with these specifications and in conformity with the lines, grades, notes, and typical sections shown in the plans

4-2 Materials

4-2.1 Asphaltic Concrete Wearing Surface

The asphaltic concrete binder and surface course shall be an S I or S III Asphaltic Concrete conforming to the requirements of Section 331 of the Florida Department of Transportation (FDOT) *Standard Specifications for Road and Bridge Construction*, latest edition containing Marshall mixes, except as herein modified. The job mix formula shall produce a minimum Marshall Stability of 1500 pounds. An approved Superpave mix Type SP as per FDOT *Standard Specifications for Road and Bridge Construction 2007, Section 334 Superpave Asphalt Concrete* may be substituted for the Marshall mixes.

4-2.2 Prime & Tack Coats

Prime and tack coats should be applied during the construction of the pavement sections in accordance with FDOT *Standard Specifications for Road and Bridge Construction 2007, Section 300 Prime and Tack Coats for Base Courses*. Before applying any bituminous material which might prevent proper bond with the existing surface for the full width of the application should be removed. Particular care should be taken in cleaning the outer edges of the strip to be treated, to ensure that the prime or tack coat will adhere. Prior to applying prime coat, the moisture content of the base should be checked to make sure that it does not exceed the optimum moisture

4-3 General Construction Requirements

The general construction requirements are as specified in FDOT *Standard Specifications for Road and Bridge Construction 2007, Section 330 General Construction Requirements*, except as may herein be modified.

4-3.1 Asphaltic Concrete Wearing Surface

The S-I or equivalent asphaltic concrete shall be placed to a compacted depth of not less than one and one-half (1-1/2) inches and shall have a minimum compacted density of 108 lbs/IN/SY. The S-III or equivalent asphaltic concrete surface shall be placed to a compacted depth of not less than three quarter (3/4) inches and shall have a minimum compacted density of 108 lbs/IN/SY. Prior to laying the mixture, the surface of the base course shall be cleaned of all loose and deleterious material.

4-4 Testing

The thickness of pavement and pavement density shall be determined by the length of cores, at two inches in diameter, taken at random points on the cross-section and at intervals of no more than two hundred (200) feet, staggered to the left, right, and on center-line along the roadway. Asphaltic Concrete Wearing Surface deficient in thickness and/or density shall be corrected to the satisfaction of the Engineer. A copy of the asphalt thickness testing results shall be provided to the Engineer. Verification thickness testing shall also be performed by the Engineer of Okaloosa County. The Contractor shall be responsible for notifying the County Engineer for asphalt inspection and thickness testing. The County Engineer shall be notified twenty-four (24) hours before the construction is ready for inspection

END OF SECTION

SECTION 5
CONCRETE PAVING

5-1 Description

The work specified in this Section consists of the construction of concrete curbs in accordance with these specifications, and in conformity with the lines, grades, dim-ensions, and notes shown in the plans.

5-2 Materials

5-2.1 Concrete

The material shall be obtained from an approved plant having an approved design mix. All work under this Section shall be of concrete having a minimum strength of 2,500 psi in 28 days. Concrete shall not be mixed or placed under adverse weather conditions

5-3 Forms

Forms for this work shall be made of either wood or metal and shall have a depth equal to the plan dimensions for the depth of concrete being deposited against them. They shall be straight, free from warp or bends, and of sufficient strength, when staked, to resist the pressure of the concrete without deviation from line and grade. Forms shall be cleaned each time they are used and shall be oiled or saturated with water prior to placing the concrete.

5-4 Foundation

Excavation shall be made to the required depth and the foundation material upon which the concrete curb is to be set shall be compacted as specified below, true to grade and cross section, and shall be moist at the time that the concrete is placed. Concrete curbs are to be constructed on a stabilized subgrade as designated in the plans. Compaction requirements for the stabilized subgrade shall be as specified in Article 2-8

5-5 Joints

5-5.1 Expansion Joints

Expansion joints shall be 1/2 inch, formed with a pre-formed joint filler. Expansion joints shall be provided at all inlets and all radius points and shall be located at intervals of 75 feet between other expansion joints or at ends of run

5-5.2 Contraction Joints

Except for machine placed items, at the option of the Contractor, joints may be formed by the use of dummy joints (either formed or sawed) or by the use of sheet metal templates. If sheet metal templates are used they shall be of the dimensions, and shall be set to the lines, shown in the plans. The templates shall be held firmly during the placing of the concrete and shall be left in place until the concrete has set sufficiently to hold its shape but shall be removed while the forms are still in place. For machine placed items, unless an alternate method is approved by the Engineer, contraction joints shall be sawed. The joints shall be sawed as soon as the concrete has hardened to the degree that excessive raveling will not occur and before uncontrolled shrinkage cracking begins. Contraction joints shall be spaced at intervals of fifteen feet except where a lesser interval is required for closure, but no section shall be less than four feet in length. Where curb is adjacent to concrete roadway pavement, the curb joints shall be made to align with the roadway joints

5-6 Placing

The concrete shall be placed in the forms and tamped and spaded to prevent honeycomb and until the top of the structure can be floated smooth and the edges rounded to the required radius.

5-7 Finishing

5-7.1 Repair of Minor Defects

The forms shall be removed within 24 hours after the concrete has been placed, and minor defects then filled with mortar composed of one part portland cement and two parts fine aggregate. Plastering will not be permitted on the face of the curb, and any rejected curb shall be removed and replaced without additional compensation.

5-7.2 Final Finish

All exposed surfaces shall be given a finish while the concrete is still green. In general, only a brush finish will be required. For any surface areas, however, which are too rough or where other surface defects make additional finishing necessary, the Engineer may require that the curb be rubbed to a smooth surface with a soft brick or wood block, with water used liberally. Also, if further necessary to provide a suitable surface, the Engineer may require additional rubbing, using a thin grout or mortar.

5-8 Curing

The concrete used for curbs shall be cured for 72 hours by a method approved by the Engineer.

END OF SECTION

SECTION 6
STORM DRAINAGE SYSTEM

6-1 Description

The work specified in this Section consists of the construction of inlets and the furnishing and installation of drainage pipe. All work shall be constructed in conformity with these specifications and the sizes and dimensions shown in the plans

6-2 Materials

6-2.1 Concrete

All inlet construction covered under this Section shall be of concrete that will have a minimum compressive strength of 3,000 psi in 28 days.

6-2.2 Reinforcement

6-2.2.1 Reinforcing steel

Shall comply with ASTM A 1015, Grade 40. Reinforcement placement shall conform to ACI 318 and with the plans.

6-2.2.2 Welded Wire Fabric shall comply with ASTM A 185

6-2.3 Gratings

Gratings and frames fabricated from structural steel shall be galvanized in accordance with the requirements of ASTM A 123 or shall be painted with two coats of prime, followed by one coat of material meeting the requirements of Federal Specifi-cation TT-E-489, Class A Black. All paint may be applied in the shop by dipping, provided that each coat is thoroughly dry before the succeeding coat is applied. These requirements do not apply when A-588 steel is used.

6-2.4 Pipe

6-2.4.1 Concrete Pipe

Concrete Pipe shall be round reinforced concrete complying with ASTM C 76, Class III, with round rubber gasket complying with ASTM C 361

6-2.4.2 Corrugated Polyethylene Pipe

Corrugated Polyethylene pipe shall be smooth interior complying with AASHTO M 294. Corrugated polyethylene pipe shall be N-12 as manufactured by Advanced Drainage Systems (ADS) or approved equal. Where so designated on the plans, corrugated polyethylene pipe shall be provided with perforations and a sock filter

6-2.5 Pre-Manufactured Drain Basins

Pre-manufactured Drain Basins shall be constructed of P.V.C. with cast iron as manufactured by Nyloplast, or approved equal. Basins shall be provided with corrugated polyethylene adapters, sized as indicated

6-3 Forms

Forms shall be of wood or metal, so designed and constructed that they may be removed without injury to the concrete. They shall be built true to line and grade and braced in a substantial and unyielding manner, and shall be approved by the Engineer before being filled with concrete

6-4 Precast Inlets

The Contractor may substitute precast inlets in lieu of cast-in-place units unless otherwise shown on the plans. When precast units are substituted the construction of such units may be in accordance with ASTM C 478. The design and fabrication of precast units shall be in accordance with the drawings

6-5 Construction Methods for Inlets

6-5.1 Excavation

Excavation shall conform with the requirements specified in Section 2 of these specifications

6-5.2 Placing and Curing Concrete

The concrete shall be placed in the forms to the depth shown in the plans and thoroughly vibrated. After the concrete has hardened sufficiently it shall be covered with suitable material and kept moist for a period of three days

6-5.3 Setting Manhole Castings

After the concrete has been cured as specified above, the frame of the casting shall be set in a full mortar bed composed of one part portland cement to two parts of fine aggregate

6-5.4 Placing Pipe

Inlet and outlet pipes shall be of the same size and kind as the connecting pipe shown in the plans. They shall extend through the walls for a distance beyond the outside surface sufficient for the intended connections, and the concrete shall be constructed around them neatly so as to prevent leakage along their outer surface. The inlet and outlet pipes shall be flush with the inside of the wall

6-5.5 Backfilling

Backfilling shall conform with the requirements specified in Section 2 of these specifications.

6-6 Laying Pipe

6-6.1 General

All pipe shall be carefully laid, true to the lines and grades given. Any pipe that is not in true alignment or which shows any settlement after laying shall be taken up and re-laid without additional compensation

6-6.2 Concrete Pipe

Concrete pipe shall be laid with hubs upgrade and tongue fully entered into the hub. When pipe with quadrant reinforcement or circular pipe with elliptical reinforcement is used, the pipe shall be installed in a position such that the manufacturer's marks designating "top" and "bottom" of the pipe shall not be more than five degrees from the vertical plane through the longitudinal axis of the pipe

6-6.3 Corrugated Polyethylene Pipe

Corrugated polyethylene pipe shall be installed in accordance with the manufac-turer's installation instructions

6-6.4 Trench Excavation

The excavation of the trench for pipe culverts and storm sewers shall be as specified in Section 2 of these specifications

6-6.5 Foundation

Where the foundation material is of inadequate supporting value a suitable foundation shall be provided, as directed by the Engineer, by the removal of unsuitable material and replacing with suitable material. Pipe shall not be laid on blocks or timbers, r on other unyielding material, except where the use of such devices is called for in the plans

6-6.6 Sealing Joints

For all concrete pipe other than side drain pipe, the pipe joints shall be sealed by the use of round rubber gaskets. The gasket and the surface of the pipe joint, including the gasket recess, shall be clean and free from grit, dirt and other foreign matter, at the time the joints are made. In order to facilitate closure of the joint, application of an approved vegetable soap lubricant immediately prior to closing of the joint will be permitted

6-6.7 Backfilling

The backfilling around the pipe shall be as specified in Section 2 of these specifications

6-7 End Treatment

The end treatment requires at each cross drain, side drain, or storm sewer pipe and shall be as shown in the plans

6-8 As-Built Drawings

The Contractor shall furnish the Engineer / City at the close of the project, with three (3) sets of marked-up construction plans depicting as-built conditions of the storm water system. Provide additional AUTOCAD as-built on state plane coordinates FL. North US Survey Feet NAD 83. The as-built plans shall show the location of storm water structures, storm water pipes and retention areas. Engineering / City approval of the storm water system will not be made until accurate and legible as-built plans are received

END OF SECTION

SECTION 7
GRASSING

7-1 Description

The work specified in the Section consists of the establishing of a stand of grass, within the limits shown on the plans by sodding or seeding, mulching, fertilizing, watering, grading and maintaining the grassed areas until the completion of the project and final acceptance.

7-2 Materials

7-2.1 Seed

Unless otherwise directed, permanent type grass seed shall be a mixture of 20 parts Bermuda seed and 60 parts Pensacola Bahia seed. Quick-growing type grass seed shall be a species which will provide an early ground cover during the particular season when planting is done and will not later compete with permanent grass. The separate types of seed used shall be thoroughly dry-mixed immediately before sowing. Seed which has become wet shall not be used. The Bermuda seed shall be an equal mixture of hulled and unhulled seed. The Pensacola Bahia seed shall be scarified seed, having a minimum active germination of 40 percent and a total germination of 85 percent. All seed shall meet the requirements of the State Department of Agriculture and Consumer Services and all applicable State Laws.

7-2.2 Sod

Sod shall be of Pensacola Bahia grass meeting the requirements of FDOT *Standard Specifications for Road and Bridge Construction 2007, Section 981 Grassing and Sodding Materials*

7-2.3 Fertilizer

The fertilizer for seeding and sodding shall meet the requirements of FDOT *Standard Specifications for Road and Bridge Construction 2007, Section 982 Fertilizer*

7-2.4 Dolomitic Limestone

Dolomitic Limestone shall be an approved product, designated for agricultural use

7-2.5 Mulch

The mulch material used for seeding shall be dry straw or hay, consisting of oat, rye, or wheat straw, or of pangola, peanut, coastal Bermuda or Bahia grass hay

7-2.6 Water

Water used in the grassing and planting operations may be obtained from any approved spring, pond, lake, stream, or municipal water system. The water shall meet the requirements of FDOT *Standard Specifications for Road and Bridge Construction 2007, Section 983 Water for Grassing*

7-3 Grassing Limits

The Contractor shall take all precautions necessary to limit the area disturbed by construction. Existing grassed areas disturbed by construction outside the limits shown on the plans, shall be re-grassed by seeding to match existing conditions, at the Contractor's expense

7-4 General Construction Methods

The general construction methods specified in FDOT *Standard Specifications for Road and Bridge Construction 2007, Section 570 Grassing (By Seeding) and Section 575 Sodding*, are to be followed.

END OF SECTION

SECTION 8
PARKING STRIPES AND MARKINGS

8-1 Description

The work specified in this Section consists of the painting of parking stripes and markings. All work shall be completed in conformity with the plans and specifications and where so referenced, to the Florida Department of Transportation Standard Specifi-cations for Road and Bridge Construction latest edition, and to the Florida Department of Transportation Roadway and Traffic Design Standard Index Drawings.

8-2 Materials

8-2.1 Paint

Paint for parking stripes shall conform to FDOT Standard Specifications for Road and Bridge Construction 2007, Section 971 Traffic Marking Materials, Code T-1 (white). Blue pavement markings shall be tinted to match shade 15180 of Federal Standards 595a.

8-3 Application and Installation

The application procedures specified in FDOT Standard Specifications for Road and Bridge Construction 2007, Section 710 Painted Pavement Markings, are to be followed.

END OF SECTION

NOT RELEASED FOR CONSTRUCTION

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VALPARAISO REALTY COMPANY

JASMINE NORTH SUBDIVISION
CITY OF VALPARAISO, FLORIDA

SPECIFICATIONS

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DRAWING NUMBER 12 OF 13
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SECTION 9
POTABLE WATER SYSTEM

9–1 Description

The work specified in this section consists of the construction of a potable water system, which includes the potable water distribution mains, valving, fire hydrants, and service piping. The potable water system shall be constructed in conformity with the plans and in accordance with these specifications, or the specifications and requirements of the Utility Company, whichever is more stringent.

9–2 Codes and Standards

The Contractor shall comply with the requirements of Chapter 62–555 of the Florida Department of Environmental Protection Rules and the applicable portions of the National Standard Plumbing Code pertaining to the installation of potable water systems. Additionally, the Contractor shall comply with the requirements of the local Fire Marshall and Utility Company regulations pertaining to fire hydrants, including hose unit threading and similar matching of connections. The Contractor shall obtain required permits and inspections and shall coordinate tapping of existing water mains with the water mains Utility Company.

9–3 Materials

9–3.1 Detector Wire

No. 14 Copper tracer wire shall be required for all non–metallic water mains and service lines. Tracer wire shall be well connected to Fire Hydrants and shall be protected during backfilling operations to prevent breakage. All splices and connections shall be made to provide both electrical and mechanical continuity.

9–3.2 Piping and Pipe Fittings

9–3.2.1 Water Mains

Water main piping shall be Polyvinyl Chloride (PVC) pipe. Water main piping 1–1/2" through 3" in size shall conform to ASTM D 2241, SDR 26 (PVC material 12454–B) with push–on joints conforming to ASTM D 3139 and elastomeric gaskets conforming to ASTM F 477. Water main piping 4" through 12" in size shall conform to AWWA C 900–89, DR 18, and shall be furnished in Cast–iron pipe equivalent outside diameters rubber gasketed joints as AWWA C 900 listed. All water main piping shall be stamped "NSF–PW". All water main fittings shall be constructed of either ductile iron or cast iron conforming to AWWA C 110 or ANSI A 21.10, respectively. All fittings shall be thin cement lined as specified in ANSI A 21.4 The exterior of the fittings shall receive a coat of hot–dip cool–tar as specified in ANSI A 21.6. All bolts, nuts, studs and other non–coated parts of joints shall be coated with asphalt or cool–tar prior to backfilling.

9–3.2.2 Water Service Tubing

Water service tubing shall be ASTM D–2737outside Diameter Controlled HDPE Tubing (CTS) rated for 250psi, MUNICIPEX Water service line as manufactured by Rehau, or approved equal.

9–3.3 Valves

9–3.3.1 Gate Valves

Gate valves shall be of cast iron body, fully bronze mounted, double–disc, and having parallel seats, with wide flanged, mechanical joint, or spigot ends depending on installation. Gate valves shall have a minimum working pressure of 175 psi, and shall comply with AWWA C 500. Gate valves shall open by counterclockwise rotation of the valve stem and shall be fitted with hub–type hand operators. Valves shall have a cast iron valve box installed concentrically over the valve. All gate valves shall be American Flow Control Resilient Wedge Gate Valves, Series 2500–1. Gate valves shall be sized and located as indicated on the plans.

9–3.3.2 Water Tapping Saddles, Sleeves and Stops

9–3.3.2.1 Tapping Saddles

Tapping saddles shall be Ford Meter Box Company Iron Series Saddles (F202 Double Strap).

9–3.3.2.2 Corporation Stops

Corporation stops shall be AWWA/CC taper thread inlet by grip joint outlet for CTS tubing, F–1000–x–G–NL style by Ford Meter Box Company.

9–3.3.2.3 Curb Stops

Curb stops shall have a grip joint inlet for CTS tubing and locking padlock wing. Curb Stops shall be Model B43–xxxW–C–NL style Ford Meter Box Company.

9–3.3.2.4 Tapping Sleeves

Tapping Sleeves shall be full circumferential seal, all 304 stainless steel with side lifter bars, Ford Meter Box Company, or approved equal.

9–3.3.3 Valve Boxes

An adjustable cast–iron valve box of suitable size shall be provided at each gate valve. The head shall be round and the lid shall have the word "WATER" cast on it. The least dimension of the shaft of the box shall be 5–1/4 inches. A 2'–0" diameter by 6" concrete collar shall be poured around the shaft at the valve box surface. The Valve Boxes shall be Clow F–2452, or approved equal.

9–3.4 Fire Hydrants

Fire hydrants shall be dry–barrel type conforming to AWWA C 502. Hydrants shall have 6 inch outlet, 5–1/4 inch valve opening, one 4–1/2 inch Storz Fitting & Cap, and two 2–1/2 inch hose connections. The size and shape of operating nuts and threads on hose and pumper connections shall be as required by the Utility Company. The hydrants shall be American Flow Control 5–1/4 B84B–5 Traffic Models

9–3.5 Anchorages

The Contractor shall provide anchorages for tees, wyes, crosses, plugs, caps, bends, valves, and hydrants as required. After installation, apply a full coat of bituminous material to the surface of any ferrous anchorages. Anchorage types are as follows:

(1) Clamps, Straps, and Washers: Steel, ASTM A 506.

(2) Rods: Steel, ASTM A 575.

(3) Rod Couplings: Malleable–Iron, ASTM A 197.

(4) Bolts: Steel, ASTM A 307.

(5) Thrust Blocks: Concrete, 2500 psi, sized as indicated.

9–4 Installation

9–4.1 Pipe Laying and Jointing

Pipe, fittings, valves and accessories shall be carefully inspected before and after installation and those found defective shall be rejected and replaced. Before placing in position, contractor to clean pipe, fittings, valves and accessories and maintain in clean condition. Cut pipe accurately to measurements established at the site and work into place without springing or forcing. Support pipe at its proper elevation and grade, taking care to secure firm and uniform support. Lay pipe so that the full length of each section of pipe and each fitting will rest solidly on the pipe bedding; excavate recesses to accommodate bells, joints, and couplings. Provide anchors and supports where necessary for fastening work into place. Concrete reaction or thrust blocks shall be applied on all tees, plugs, caps and bends deflecting 22–1/2 degrees or more. Keep trenches free of water until joints have been properly made. At the end of each days work, close open ends of pipe temporarily with wood blocks or bulkheads. Do not lay pipe when conditions of trench or weather are unsuitable. In addition to the above, PVC pipe and fittings installation shall conform to the requirements of UNI B–3.

Water mains shall be laid at least 6 feet horizontally from any existing or proposed sewer. The distance shall be measured edge to edge. In cases where it is not practical to maintain a 6 foot separation, and when approved by D.E.P. and the Engineer, the water main may be installed closer to the sewer, provided that the water main is laid in a separate trench or on an undisturbed earth shelf located a minimum of 3 feet from the sewer at such an elevation that the bottom of the water main is at least 6 inches above the top of the sewer.

Water mains crossing over sanitary sewer mains shall be laid to provide a minimum vertical distance of 6 inches (12 inches preferred) between the invert of the upper pipe and the crown of the lower pipe. Where this minimum separation cannot be maintained, the crossing shall be arranged so that the sewer pipe joints and water main joints are equidistant from the point of crossing with no less than 10 feet between joints. Alternatively, the sewer main may be placed in a sleeve to obtain the equivalent of the required 10–feet separation.

Excavation and backfilling of water main trenches shall conform to Section 2 of these specifications. Provide a minimum of 36 inches of cover over water mains and a maximum depth of 48 inches. Depths beyond 48 inches must be pre–approved by the Utility Company. Sufficient separation between all utilities shall be provided such for all utilities within the right of way, to avoid damage during installation and also to allow subsequent repair to be performed.

9–4.2 Water Service Tubing

Water service tubing crossing paved areas shall be installed within a 2" Schedule 40 P.V.C. chase pipe.

9–4.3 Water Service Taps and Service Connections

Water service taps and service connections shall be installed in locations so as not to be covered by the anticipated construction of asphalt or concrete pavement of future driveways or roads, etc.

9–4.4 Gate Valves

Install gate valves in accordance with the requirements of AWWA C 600 for valve–and–fitting installation. Valve boxes shall be placed concentrically over the valve operating nut.

9–4.5 Fire Hydrants

Install fire hydrants in accordance with the requirements of AWWA C 600 and AWWA M 17, "Installation, Operation, and Maintenance of Fire Hydrants".

9–4.6 Connection to Existing Water System

Coordinate and make connections to the existing water system in a manner approved by the Utility Company, and with a minimum interruption of service on the existing system. Proof of a Department of Environmental Protection clearance letter will be required prior to connections to the water system. The City of Niceville Water Department is the sole agency allowed to make a main line tap into an existing water main. However, it is the responsibility of the customer to correctly size both the service tap and service line. Also, it is up to the Utility Contractor requesting the tap to provide the appropriate tapping sleeve, implement appropriate maintenance of traffic plan (if required), do all excavating (in accordance with the Florida Trench Safety Act), to install the piping, valve and meter boxes or meter vaults, and to complete all backfilling and grade restoration. The Utility Contractor is to clearly mark the main at the location of the desired tap. The Utility Contractor must be present at the time of the tap. There will be a fee for this main line tapping service.

9–5 Field Testing

9–5.1 General

Conduct piping tests before joints are covered, and after thrust blocks have sufficiently hardened. Fill pipeline with water 24 hours prior to testing, and apply test pressure to stabilized system.

9–5.2 Hydrostatic Tests

9–5.2.1 Pressure During Test

After the pipe has been laid and backfilled as specified, each valved section of newly laid pipe shall, unless otherwise specified, be subjected to a hydrostatic pressure equal to 150 psi. The Contractor shall record the testing by the use of a pressure recording gauge, and after all testing is complete, the recording shall be turned over to the Engineer for his files.

9–5.2.2 Duration of Pressure Test

The duration of each pressure test shall be at least 24 hours.

9–5.2.3 Procedure

Each section of pipe shall be slowly filled with water and the specified test pressure, measured at the lowest point of the elevation, shall be applied by means of a pump connected to the pipe in a satisfactory manner. The pump, pipe connection, gauges meter, and all necessary apparatus shall be furnished by the Contractor. The test shall be applied to each valved section in order to check the leakage through all valves.

9–5.2.4 Expelling Air Before Test

Before applying the specified test pressure, all air shall be expelled from the pipe. To accomplish this, taps shall be made, if necessary, at points of highest elevation and afterwards tightly plugged.

9–5.2.5 Definition of Leakage

Leakage is defined as the quantity of water to be supplied into the newly laid pipe, or any valved section of it, necessary to maintain the specified leakage test pressure after the pipe has been filled with water and the air expelled.

9–5.2.6 Permissible Leakage

Suitable means shall be provided by the Contractor for determining the quantity of water lost by leakage under normal operating pressure. No pipe installation will be accepted until or unless this leakage (evaluated at 150 psi) is less than the figure stated below:

	Gal's/1000'/24 Hrs.	Gal's/1000'/1 Hr.
2"	3.8 gal.	0.16 gal.
3"	5.7 gal.	0.24 gal.
4"	7.6 gal.	0.32 gal.
6"	11.4 gal.	0.47 gal.
8"	15.2 gal.	0.53 gal.
10"	18.9 gal.	0.79 gal.

9–5.2.7 Variation From Permissible Leakage

Should any test of combined sections of pipe laid disclose leakage greater than the specified limit, the Contractor shall, at his expense, locate and repair the defective joints until the leakage is within the specified allowance.

9–5.2.8 Water for Testing

Water for testing shall be provided by the Contractor at his expense.

9–5.2.9 Time for Making Test

Pipe may be subjected to hydrostatic pressure, inspected, and tested for leakage at any convenient time after partial completion of backfill. The Contractor may test the system with joints exposed or with backfilling complete at his option. The Engineer and the Utility Company shall be notified at least 24 hours before beginning testing.

9–5.3 Operating Tests

Open and close all valves and hydrants under system water pressure. Check dry barrel hydrants for proper drainage.

9–6 Disinfection

Flush and disinfect the new potable water system in accordance with AWWA C 601 to the satisfaction of the Engineer and the Utility Company. In the process of chlorinating water, all valves or other appurtenances shall be operated while the pipe line is filled with the chlorinating agent.

9–7 Final Flushing and Testing

Following disinfection, all treated water shall be thoroughly flushed from the newly laid pipe system at its extremities until the replacement water throughout its length shall upon test, both chemically and bacteriologically, be approved by the Florida Department of Environmental Protection. The Contractor shall be responsible for arranging and paying for the test samples.

9–8 Alterations to Specifications or Approved Plans

Any alterations, changes, or deviations from the specifications or the approved plans must be re–submitted to the Building Inspector's Review Office for review/approval by the Technical Review Committee before any further installation is conducted.

9–9 As–Built Certification

In accordance with the Rules of Florida Department of Environmental Protection, Chapter 62–555, the Engineer of record will be responsible for observation of the construction of the potable water system to assure compliance with plans and specifications, and said Engineer will report to DEP upon completion of the construction and cleaning and disinfecting described above before the system can be placed into service. The Contractor shall notify the Engineer a minimum of 24 hours before beginning construction on the potable water system.

9–10 As–Built Drawings

The Contractor shall furnish the Engineer at the close of the project, with three (3) sets of marked–up construction plans depicting as–built conditions of the potable water system. Provide additional AUTOCAD as–built on state plane coordinates FL. North US Survey Feet NAD 83. The as–built plans shall show the location of water services, water valves, and fire hydrants. Engineering approval of the potable water system will not be made until accurate and legible as–built plans are received.

END OF SECTION

SECTION 10
SANITARY SEWAGE SYSTEM

10–1 Description

The work specified in this Section consists of the construction of a gravity sanitary sewer collection system, including sanitary sewer collection lines, service lines, sanitary sewer manholes, and other appurtenances, in conformity with the plans and in accordance with these specifications, or the specifications and requirements of the City of Niceville Water and Sewer Department (Utility Company), whichever is more stringent.

10–2 Codes and Standards

The Contractor shall comply with the requirements of Chapter 62–604 of the Florida Department of Environmental Protection rules and the applicable portions of the National Standard Plumbing Code and Ten State Standards, pertaining to the installation of gravity sanitary sewer collection systems. The Contractor shall obtain required permits and inspections from the Utility Company.

10–3 Materials

10–3.1 Gravity Sewer Collection Lines and Fittings

Gravity sewer collection line piping and fittings (6 inches through 12 inches) shall be Polyvinyl Chloride (PVC) conforming to ASTM D 3034, Type PSM, SDR 35. Jointing shall be accomplished by means of elastomeric gasket joints conforming to ASTM D 3212. Gasket material shall be suitable for use with domestic sewage and shall conform to ASTM F 477.

10–3.2 Sewer Service Lines

Sewer service line piping and fittings (4 inches and smaller) shall be Polyvinyl Chloride (PVC) conforming to ASTM D 2466. Solvent cement for jointing piping shall conform to ASTM D 2565.

10–3.3 Manholes

Sanitary sewer manholes shall be constructed of precast reinforced concrete, sized as indicated, and complying with ASTM C 478. Manhole tops shall be eccentric cone type and manhole bases shall be base riser sections with integral floors. Precast manholes shall be provided with ductile iron steps integrally cast into the manhole walls and with resilient pipe connectors complying with ASTM C 923.

Sanitary sewer manhole frames and covers shall be of heavy–duty cast iron construction, sized as indicated. The manhole cover shall be of indented top design with lettering cast into the top reading "SEWER". Manhole frames and covers shall be Vulcan Foundry No. VM–37, Neenah Foundry No. R–1600, or approved equal. Manhole frames and covers are to be coated with coal–tar epoxy.

10–3.4 Cleanouts

At the property line or point of connection to the city sewer service tap, a 4" sewer cleanout shall be installed and cut to ground level at finish grade. A cement collar shall be installed around the cleanout. The concrete collar should be pre–casted or poured in place with an outside diameter of no less than 16" and a center open diameter of no less than 6"–8" and the concrete thickness should be no less than 4".

10–4 Installation

10–4.1 Gravity Collection and Service Lines

Each section of pipe shall be carefully inspected before and after it is installed and defective pipe shall be rejected and removed. Sewer lines shall be laid beginning at the low point of the system, true to the grade and alignment indicated, with unbroken continuity of invert. Clear interior of piping of dirt and other superfluous material as work progresses. Maintain swab or drag in line and pull past each joint as it is completed. Open ends of pipe at the end of each day's work shall be closed temporarily with wood blocks or bulkheads. In addition to the above, PVC gravity sewer lines shall be installed in accordance with UNI–B–5.

Sewers shall be laid at least 10 feet horizontally from any existing or proposed water main. The distance shall be measured edge to edge. In cases where it is not practical to maintain a ten foot separation, and when approved by D.E.P. and the Engineer, the sewer may be installed closer to the water main, provided that the water main is in a separate trench or on an undisturbed earth shelf located on one side of the sewer at an elevation so that the bottom of the water main is at least 18 inches above the top of the sewer.

Sewers crossing under water mains shall be laid to provide a minimum vertical distance of 18 inches between the invert of the upper pipe and the crown of the lower pipe. Where this minimum separation cannot be maintained, the crossing shall be arranged so that the sewer pipe joints and water main joints are equidistant from the point of crossing with no less than 10 feet between any two joints. Alternatively, the sewer main may be placed in a sleeve to obtain the equivalent of the required 10–feet separation.

Excavation of pipe trenches shall conform to Section 2 of these specifications.

10–4.2 Sewer Manholes

Sanitary sewer manholes shall be installed in accordance with ASTM C 891. Where manholes occur in pavements, set top of frames and covers flush with the finished surface. Elsewhere, set tops 3" above finished surface, unless otherwise indicated. Grout inverts of manholes to provide continuity of grade and direction change through manhole. The exterior of the manhole shall be given two coats of Koppers 300M, or equal, before backfilling operation.

Sewer Manholes shall be coated inside and outside with a self–priming heavy duty cold applied coal tar material similar and equal to KOPPERS "Bitumastic Super Service Black". Provide a minimum of two (2) coats at a rate of 50 to 60 square feet per gallon per coat.

10–5 Backfilling

Backfill pipe trenches and ground structures in accordance with Section 2 of these specifications.

10–6 Inspection and Testing

Inspect piping to determine whether line displacement or other damage has occurred. Make inspections after lines between manholes, or manhole locations, have been installed approximately 2' of backfill is in place, and again at the completion of the project. If inspection indicates poor alignment, debris, displacement, or other defects, correct such defects and reinspect. Prior to initial and final acceptance of the project, all sewer mains shall be internally video inspected and a written report and a color video in flash drive or DVD format submitted to the CITY for their approval and records prior. The work shall include television inspection of sewer mains from center to center of each manhole, a digital or audio commentary on location with distance encoding, a written or digital report, still photographs of problem areas and all manhole connections. Prior to video inspection, the CONTRACTOR shall flush all sewers with water sufficient in volume to obtain free flow through each line. During flushing, debris shall not enter any pump station wet well. Flushing and cleaning of the line prior to the inspection shall remain the responsibility of the CONTRACTOR. The CONTRACTOR shall give 72 hour notice to the CITY before video inspecting any segment; the CITY shall have the option to witness all or any portion of the inspection. If the CITY finds that any completed line or part thereof fails, the CONTRACTOR shall be required to repair the sewer line until it meets specifications of the CITY.The CITY will require additional video inspection for the sewer line segments in question. All costs of this work shall be at the expense of the CONTRACTOR.

Leakage tests may include appropriate water or low pressure air testing. The leakage outward or inward (exfiltration or infiltration) shall not exceed 50 gallons per inch of pipe diameter per mile per day for any section of the system. An exfiltration or infiltration test as appropriate shall be performed with a minimum positive head of two (2) feet. The water test, if used, shall be run continuously for a period of not less than three (3) hours, with weir reading taken at 20 minute intervals. The air test, if used, shall, as a minimum, conform to the test procedure described in ASTM F1417–92 (1998) "Standard Test Method for Installation Acceptance of Plastic Gravity Sewer Lines Using Low–Pressure Air". All test times shall be calculated using Ramseier's equation T = 0.085 (DK/Q) Where

T = Shortest time, in seconds allowed for the air pressure to drop 1.0 psig

K = 0.000419 DL, but not less than 1.0

Q = 0.0015 cubic feet/minute/ square feet of internal surface

D = nominal pipe diameter in inches

L = Length of pipe being tested in feet

For more efficient testing of long test sections and /or sections of larger diameter pipes a timed pressure drop of 0.5 psig may be used in lieu of the 1.0 psig drop. If 0.5 psig pressure drop is used, appropriate required test times shall be exactly half as long as those obtained using Ramseier's equation for T cited above.

The testing methods provided should take into consideration the range in groundwater elevations projected and the situation during the test.

The test shall be made by the Contractor with the Utility Company representative being present to verify the test accuracy. The Engineer and the Utility Company shall be notified 24 hours in advance. Where infiltration occurs in excess of the specified amount, the defective pipe or joints shall be located and repaired at the expense of the Contractor. If the defective portions cannot be so located, the Contractor, at his own expense, shall remove and reconstruct as much of the original work as necessary to obtain a sewer system within the allowable infiltration limits.

The sewer or sewers shall be pumped out and normal infiltration conditions prevail before testing shall be started.

All sewer mains and services shall be pressure tested as specified in this section or as required by the Florida Department of Environmental Protection and the Utility Company.

10–6.2 Sewer Manholes

Sanitary sewer manholes shall be inspected for water tightness prior to being placed into service. Leakage tests may include appropriate water or air pressure testing. the leakage outward or inward (exfiltration or infiltration) shall not exceed 3/4 gallons per hour for any manhole in the system. When an exfiltration or infiltraton test is performed on the sewer mains and services, the Contractor may elect to include the sewer manholes in the section of sewer being tested. The air test if used shall conform to test procedures described in ASTM C1244–93 (2000) " Standard Test Method for Concrete Sewer Manholes by the Negative Air Pressure Test". The testing methods should take into consideration the range of groundwater elevations projected and the situation during the test.

The manhole shall be pumped out and normal infiltration conditions prevail before testing shall be started.

All sewer manholes shall be pressure tested as specified in this section or as required by the Florida Department of Environmental Protection and the City of Niceville.

The tests shall be made by the Contractor. The Engineer and the Utility Company shall be notified 24 hours in advance. Where exfiltration/ infiltration occurs in excess of the specified amount, the defective manhole shall be located and repaired at the expense of the Contractor. If the defect cannot be so located, the Contractor, at his own expense, shall remove and reconstruct as much of the original work as necessary to obtain a manhole within the allowable limits.

10–7 As–Built Certification

In accordance with the Rules of the Florida Department of Environmental Protection, Chapter 62–604, the Engineer of record will be responsible for observation of the construction of the sanitary sewer collection system to assure compliance with the plans and specifications, and said Engineer will report to DEP upon completion of the construction before the system can be placed into operation. The Contractor shall notify the Engineer a minimum of 24 hours before beginning construction on the sewer collection system.

10–8 As–Built Drawings

The Contractor shall furnish the Engineer at the close of the project, with three (3) sets of marked–up construction plans depicting as–built conditions of the sanitary sewer system. Provide additional AUTOCAD as–built on state plane coordinates FL. North US Survey Feet NAD 83. The as–built plans shall show the location of sanitary sewer services and the location and elevation of sanitary sewer manholes. Engineering approval of the sanitary sewer system will not be made until accurate and legible as–built plans are received.

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VALPARAISO REALTY COMPANY

JASMINE NORTH SUBDIVISION
CITY OF VALPARAISO, FLORIDA
SPECIFICATIONS
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JOB: E15020
DATE: 04-06-2016
DESIGNED: MS/MZ
DRAWN: MS

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IF NOT ONE INCH ON THIS SHEET ADJUST SCALES ACCORDINGLY

DRAWING NUMBER
13 OF 13
SHEET NUMBER
C12

JENKINS ENGINEERING, INC.
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File: J:\Eng-proj\2015\E15020 VRC Jasmine Ave North SD\dwg\E15020 Jasmine North-8A.DWG Last Save: 5/4/2016 9:40 AM Last saved by: Mscotka
Last plotted by: Marek Scotka Plot Style: GCT2011.ctb Plot Scale: 1:2.58 Plot Date: 5/4/2016 9:42 AM Plotter used: None

ORDINANCE NO. 671

AN ORDINANCE OF THE CITY OF VALPARAISO, FLORIDA, ESTABLISHING PURSUANT TO SECTION 196.1995, FLORIDA STATUTES, FOR TEN YEARS, THE AUTHORITY TO GRANT CERTAIN ECONOMIC DEVELOPMENT AD VALOREM TAX EXEMPTIONS; PROVIDING THAT APPROVAL OF THE EXEMPTION AUTHORITY OCCURRED BY REFERENDUM OF THE VOTERS OF THE CITY ON MARCH 15, 2016; PROVIDING FOR GRANTING OF CERTAIN ECONOMIC DEVELOPMENT AD VALOREM TAX EXEMPTIONS; PROVIDING FOR SEVERABILITY; AND PROVIDING FOR AN EFFECTIVE DATE.

WHEREAS, the City of Valparaiso of Okaloosa County, Florida, is a municipal corporation duly created and existing pursuant to the Constitution and laws of the State of Florida; and,

WHEREAS, it is the intent of the City to establish and further a sound business and industrial tax base in the City of Valparaiso, of which said tax base will benefit all residents in the City by better broadening the distribution of the burden of said taxes; and,

WHEREAS, this intent and purpose is accomplished by attracting those businesses and industries that pay greater than the average annual Okaloosa County wage to locate in the City; and,

WHEREAS, the Legislature of the State of Florida enacted Section 166.021(8) of the Florida Statutes to facilitate the growth and creation of business enterprises in the municipalities of the State of Florida; and,

WHEREAS, the City desires to participate in and promote economic development incentive programs that will assist in accomplishing the purposes of attracting targeted industries with value-added employment opportunities.

NOW, THEREFORE, BE IT ORDAINED BY THE CITY COMMISSION OF THE CITY OF VALPARAISO, FLORIDA:

SECTION 1. TITLE.

This ordinance shall be known and may be cited as the "City of Valparaiso Economic Development Incentive Ordinance."

SECTION 2. PURPOSES AND FINDINGS OF FACT.

The preceding recitals are true and correct, and are incorporated herein. It is the intent and purpose of this ordinance to provide the authority and mechanisms within the parameters of federal, state and local laws, to allow the expenditure of public funds for promotions and incentives to attract industries or businesses to locate within the City and through expansion of certain existing industries or businesses, particularly when such industries and businesses pay above average annual wage to locate or expand in the City.

SECTION 3. DEFINITIONS.

For the purpose of this ordinance, the following definitions shall apply.

A. *Above average annual wage* means one hundred ten percent (110%) or greater of the average annual wage for Okaloosa County.
B. *Average annual wage* means the average annual wage paid to workers in Okaloosa County as published by the State of Florida, Department of Economic Opportunity, Bureau of Labor Market Statistics, for the most current year published.
C. *Applicant* means the industrial or business entity seeking to be verified by the City as an eligible industry or business pursuant to this ordinance.
D. *Attributable tax increment* means the increased increment in the City's ad valorem and tangible property tax base attributable or apportionable to new development, redevelopment or expansion of an industry or business.
E. *Business or industry* means any business entity authorized to do business in the state, county and the city, including all members of an affiliate group of corporations as defined in Section 220.03 (1)(b) of the Florida Statutes.
F. *Expansion of an Existing Business* means the expansion of a business or industry resulting in the increase in jobs and wages as defined herein.
G. *Full-time* means any full-time equivalent position(s) including, but not limited to, positions obtained through co-employment under a professional employer organization agreement.
H. *Jobs* means full-time equivalent positions, as such terms are consistent with terms used by the Florida Department of Economic Opportunity and U.S. Department of Labor for purposes of unemployment compensation tax administration and employment estimation. Jobs shall not include temporary construction jobs involved with the construction of facilities for the project or any jobs which have been previously included in any incentive benefit provided pursuant to this ordinance.
I. *New business* means a business or industry which does not exist within the City, first begins its operations on a site located within the City and is clearly separate from any other operation owned by the same business or industry.
J. *New job* means the addition of a new full-time job which is wholly based in the City and which previously did not exist as part of the City's total job base.
K. *Project* means the creation of a new business or industry or the expansion of an existing business or industry.

SECTION 4. ECONOMIC DEVELOPMENT AD VALOREM TAX EXEMPTION.

Subject to the provisions of this ordinance, certain new businesses or expansions of an existing business or industry due to relocation to the City or through expansion of existing facilities within the City may be granted an ad valorem property tax exemption of up to 100% of the increased value added to the tax roll for up to ten (10) years.

To comply with statutory requirements, eligible applicants must meet the job creation threshold of 10 full-time (or full-time equivalent) jobs for a business that manufactures, processes, compounds, fabricates or produces for sale items of tangible personal property at a fixed location and which comprises an industrial or manufacturing plant; 25 full time (or full-time equivalent) jobs for other industries with a sales factor less than 0.50 for each year that the exemption is claimed; 50 full time (or full-time equivalent) jobs for an office use provided that such business or organization office first begins operation on a site clearly separate from any other commercial or industrial operation owned by the same business or organization; or any

business or organization located in an enterprise zone or brownfield area that first begins operation on a site clearly separate from any other commercial or industrial operation owned by the same business or organization.

The City Commission, in its sole discretion, shall determine and set guidelines to insure continued consistency with statutory requirements and to integrate local input within the guidelines to achieve the desired results of offering this incentive. No precedent shall be implied or inferred by the granting of an exemption to a New Business or Expansion of an Existing Business.

SECTION 5. ADMINISTRATION AND PROCEDURES.

A supporting Policy & Procedures Manual to implement the exemptions and incentives provided for in this ordinance shall be developed upon adoption of this ordinance.

SECTION 6. PERFORMANCE DEMONSTRATION AND REVIEW.

Following adoption of an ordinance granting an Economic Development Ad Valorem Tax Exemption or following execution of a grant of exemption agreement as required by this ordinance, the City may periodically conduct site visits and audits of the New Business or Expansion of an Existing Business to ensure that the business is satisfactorily performing under the ordinance or grant of exemption agreement and this ordinance. As a prerequisite of receiving an exemption or incentive from the City in year one and each year thereafter, the New Business or Expansion of an Existing Business shall demonstrate:

A. That it has fully satisfied all terms and conditions contained in the agreement as described in this ordinance continuing through the subject funding period;
B. That it has not been, nor is, in breach or default of the grant of exemption agreement or any other incentive agreement with the City pursuant to this ordinance; and,
C. That it has created the number of eligible new jobs promised within the time frames set forth below:
    1. For any new construction or expansion of a building or facility to be used and occupied by the New Business or Expansion of an Existing Business, the New Business or Expansion of an Existing Business shall demonstrate that it has created the number of eligible new jobs promised no later than twelve (12) months from receiving the certificate of occupancy for its primary building or facility, or from the date of completing its multi-year expansion.
    2. For any existing building or facility, the New Business or Expansion of an Existing Business shall demonstrate that it has created the number of eligible new jobs promised no later than twelve (12) months from moving into its building or facility and commencing operations, or from the date of completing its multi-year expansion.

SECTION 7. CESSATION OF OPERATIONS DURING EXEMPTION TERM.

In the event the New Business or Expansion of an Existing Business ceases its operations, closes its business, moves out of the City, loses its status as a New Business or Expansion of an Existing Business at any time during the ordinance or agreement period, or is in breach or default of any ordinance or grant of exemption agreement with the City under this ordinance, then the New Business or Expansion of an Existing Business shall forfeit any and all funding/incentives which may be remaining under its agreement and any other incentive pursuant to this ordinance.

SECTION 8. INCENTIVES NON-EXCLUSIVE.

The incentive in this ordinance is not exclusive and eligible businesses or industries may apply for as many incentives as they qualify for under other resolutions, ordinances or provisions of the Code. Further, the incentive in this ordinance does not preclude an industry or business from applying or qualifying in other federal, state or local programs. City Commission, in its sole discretion, reserves the right to limit or restrict the number of applicants and the total number and dollar amount of all grants of exemption, deferrals and incentives under this ordinance which may be awarded in any given City fiscal year.

SECTION 9. REFERENDUM ELECTION.

The authority to grant economic development tax exemptions established by this ordinance was approved by a majority of the electors of the City of Valparaiso voting in the referendum election on March 15, 2015.

SECTION 10. INCORPORATION INTO CODE OF ORDINANCES.

This ordinance shall be incorporated into the City of Valparaiso's Code of Ordinances and any section or paragraph number or letter and any heading may be changed or modified as necessary to effectuate the foregoing.

SECTION 11. CONFLICTING PROVISIONS.

Special Acts of the Florida Legislature applicable to the incorporated area of the City of Valparaiso, City Ordinances and City Resolutions, or parts, thereof, in conflict with the provisions of this ordinance are hereby superseded by this ordinance to the extent of such conflict.

SECTION 12. SEVERABILITY.

Each separate provision of this ordinance is deemed independent of all other provisions herein so that if any portion or provision of this ordinance is declared invalid, all other provisions thereof shall remain valid and enforceable.

SECTION 13. EFFECTIVE DATE.

This ordinance shall take effect immediately upon approval on second reading by City Commission and signature of the Mayor.

ADOPTED IN SESSION THIS ____ DAY _____ OF 2016

_____
John B. Arnold, Jr.
Mayor

ATTEST:

_____
Tammy Johnson, CMC
City Clerk